## CITY OF BRYAN, TEXAS

## THE MODS

### PRIVATE IMPROVEMENTS

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#### PROJECT NOTES

ENGINEER'S CERTIFICATION: I CERTIFY THAT THESE PLANS WHICH BEAR MY SEAL HAVE BEEN PREPARED BY ME OR UNDER MY DIRECT SUPERVISION AND ARE IN COMPLIANCE WITH ALL APPLICABLE CITY, STATE AND FEDERAL REQUIREMENTS. THE PROPOSED IMPROVEMENTS SHOWN IN THESE PLANS WILL NOT IMPEDE THE FLOW OF SURFACE WATERS FROM HIGHER ADJACENT PROPERTIES, WILL NOT ALTER THE NATURAL FLOW OF SURFACE WATERS SO AS TO DISCHARGE THEM UPON ADJACENT PROPERTIES AT A MORE RAPID RATE OR IN A DIFFERENT LOCATION, AND WILL NOT CONCENTRATE FLOWS OF SURFACE WATERS IN A MANNER WHICH EXCEEDS THE CAPACITY OF THE RECEIVING WATERCOURSE. THIS CERTIFICATION DOES NOT APPLY TO ANY EXISTING IMPROVEMENTS ON THE SUBJECT

FIELD VERIFY ALL EXISTING CONDITIONS AND ELEVATIONS INCLUDING PAVEMENT AND UTILITY TIE-INS PRIOR TO CONSTRUCTION. NOTIFY ENGINEER OF ALL DISCREPANCIES PRIOR TO BEGINNING ANY WORK.

TEXAS DEPARTMENT OF LICENSING AND REGISTRATION (TDLR) NUMBER REQUIRED FOR ALL PROPOSED COMMERCIAL BUILDINGS. \*\*IF TDLR NUMBER IS NOT PRESENT, CLIENT IS RESPONSIBLE FOR ACQUIRING REGISTRATION NUMBER PRIOR TO CONSTRUCTION.

WETLAND NOTE:

THESE PLANS WERE PREPARED WITHOUT THE BENEFIT OF AN ENVIRONMENTAL OR OTHER WETLANDS STUDY, L SQUARED ENGINEERING IS NOT AN ENVIRONMENTAL ENGINEERING FIRM AND

DOES NOT HAVE THE ABILITY TO DETERMINE ENVIRONMENTAL OR WETLAND IMPACTS. THE CLIENT AND/OR OWNER SHALL BE RESPONSIBLE FOR ANY SUCH STUDY AND NOTIFY ENGINEER IF ANY RESULTING CHANGES ARE NEEDED PRIOR TO CONSTRUCTION.

APPROXIMATELY 6.299 ACRES BEING LOT 17 IN BLOCK 14 OF BRIARCREST PARK ADDITION. THE SUBJECT TRACT IS LOCATED IN

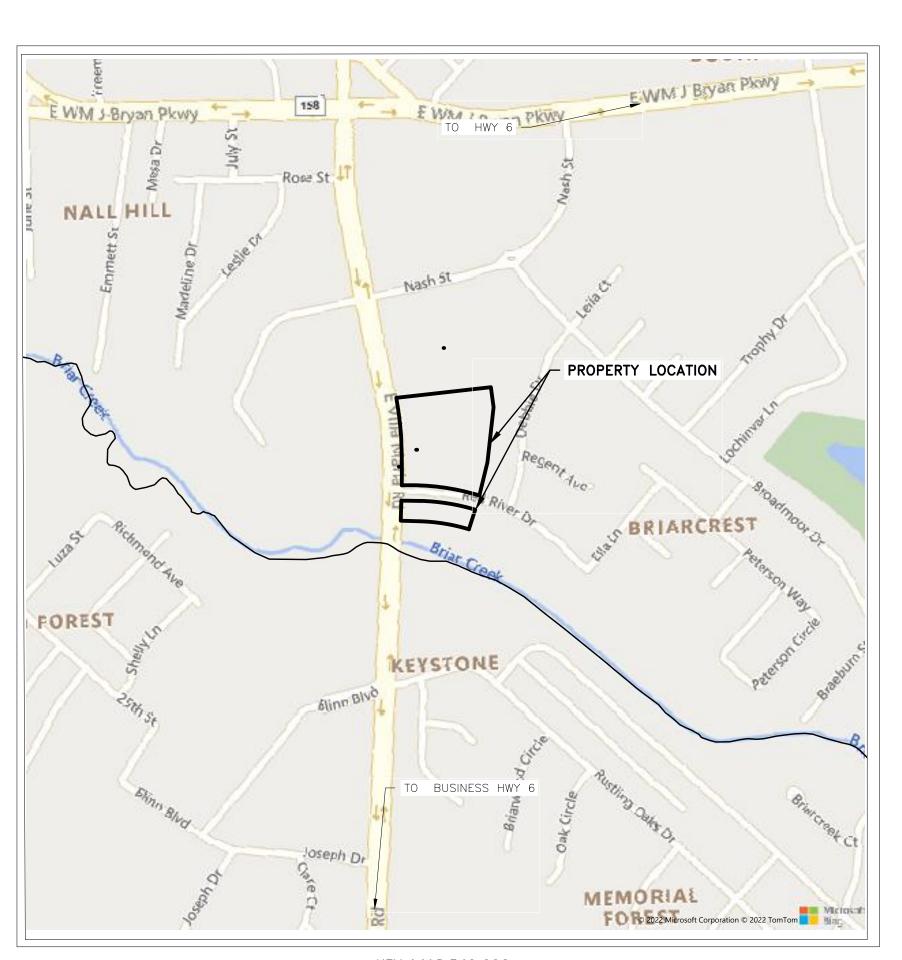
THE CITY OF BRYAN. (BRAZOS COUNTY PROPERTY ID: 30053).

THE BEARINGS SHOWN HEREON ARE BASED ON NAD.83, TEXAS CENTRAL ZONE.

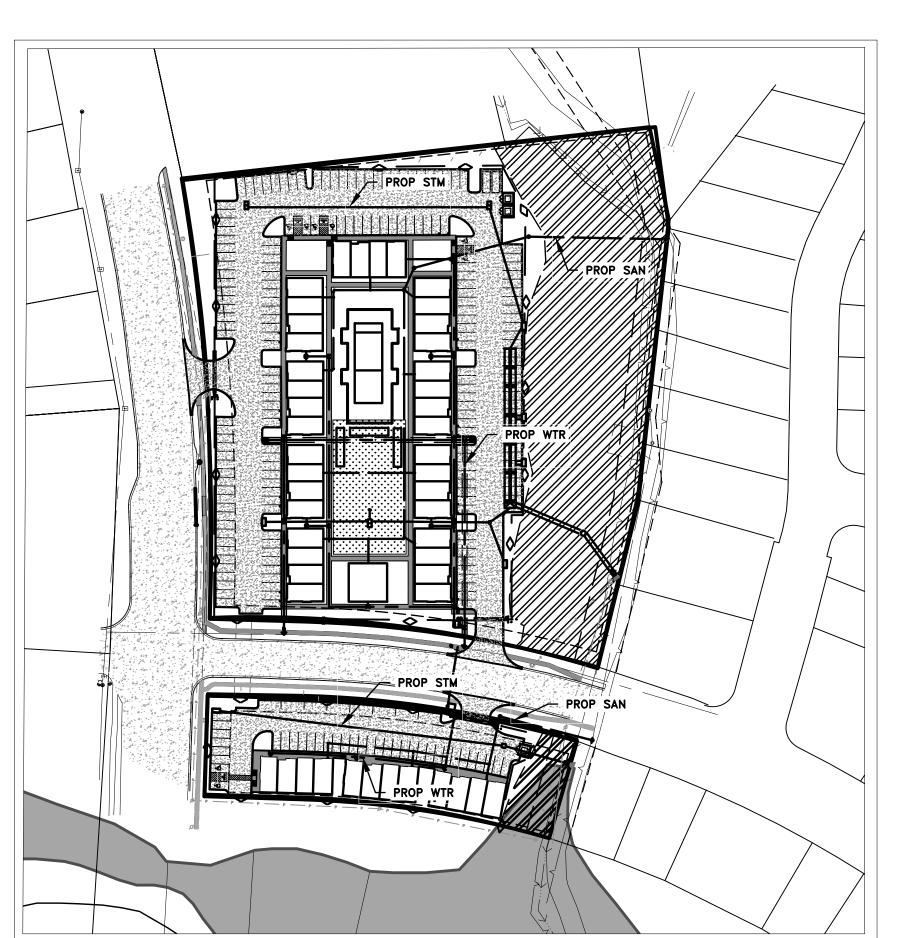
THE PROPERTY IS LOCATED WITHIN AN AREA HAVING A ZONE DESIGNATION X BY THE FEDERAL EMERGENCY MANAGEMENT AGENCY (FEMA), ON FLOOD INSURANCE RATE MAP NO. 48041C0215F, WITH A DATE OF IDENTIFICATION OF APRIL 2, 2014, IN BRAZOS COUNTY, STATE OF TEXAS, IN WHICH THE PROPÉRTY IS SITUATED.

SURVEY PROVIDED BY \_\_\_\_\_ DATED \_\_\_\_.

CONTRACTOR TO VERIFY EXISTING CONDITIONS PRIOR TO ANY WORK AND NOTIFY ENGINEER OF ANY DISCREPANCIES.



KEY MAP 549-228 SCALE: 1" = 500'



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DRAWING ISSUE												
#	DATE	ВҮ	* COMMENT									
1	9/27/22	JTW	FOR PERMIT									

PROJECT	10766	TDLR	**
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	JONATHAN	***************************************	

PROJECT MAP SCALE: 1" = 100'

CONFLICTS WITH THE APPROPRIATE GOVERNING AGENCY.

1. MATERIALS, CONSTRUCTION AND TESTING TO BE IN ACCORDANCE WITH THE GOVERNING ENTITY'S ORDINANCES AND SPECIFICATIONS, LATEST PRINTING AND AMENDMENTS THERETO.

2. CONTRACTOR TO OBTAIN ALL DEVELOPMENT AND CONSTRUCTION PERMITS REQUIRED BY ALL ENTITIES AT HIS EXPENSE PRIOR TO COMMENCEMENT OF WORK.

3. CONTRACTOR SHALL GIVE NOTICE TO ALL AUTHORIZED INSPECTORS, SUPERINTENDENTS OR PERSONS IN CHARGE OF PRIVATE AND PUBLIC UTILITIES OR RAILROADS AFFECTED BY HIS OPERATIONS 48 HOURS PRIOR TO COMMENCEMENT OF WORK IN STREET RIGHTS-OF-WAY OR EASEMENTS.

4. ALL EXISTING UNDERGROUND UTILITIES SHOWN ARE NOT GUARANTEED TO BE COMPLETED OR DEFINITE, BUT WERE OBTAINED FROM THE BEST INFORMATION AVAILABLE. CONTRACTOR HAS SOLE RESPONSIBILITY FOR FIELD VERIFICATION OF ALL EXISTING FACILITIES SHOWN ON DRAWINGS. CONTRACTOR SHALL COORDINATE ALL

5. THE LOCATION OF LUFKIN-CONROE TELEPHONE EXCHANGE OR AT&T COMPANY, ENTEX, AND ENTERGY-GSU (GULF STATES UTILITIES) UTILITIES ARE SHOWN IN AN APPROXIMATE WAY ONLY. THE CONTRACTOR SHALL REQUEST THE EXACT LÓCATION OF THESE FACILITIES BY CALLING THE UTILITY COMPANIES, AT LEAST 48 HOURS BEFORE COMMENCING WORK. THE CONTRACTOR IS FULLY RESPONSIBLE FOR ANY AND ALL DAMAGE WHICH OCCURS DUE TO HIS FAILURE TO REQUEST THE LOCATION AND PRESERVATION OF THESE UNDERGROUND FACILITIES. ANY DAMAGE TO EXISTING FACILITIES INCURRED AS A RESULT OF CONSTRUCTION OPERATIONS WILL BE REPAIRED BY THE CONTRACTOR AT HIS OWN EXPENSE.

6. TEXAS LAW ARTICLE 1436C, PROHIBITS ALL ACTIVITIES IN WHICH PERSONS OR EQUIPMENT MAY COME WITHIN 6 FEET OF ENERGIZED OVERHEAD POWER LINES, AND FEDERAL REGULATION, TITLE 29, PART 1910.130(1) AND PART 1926.440 (A) (15) REQUIRE A MINIMUM CLEARANCE OF 10 FEET FROM THESE FACILITIES. THE ABOVE LAWS CARRY BOTH CRIMINAL AND CIVIL LIABILITIES, WITH CONTRACTORS AND OWNERS BEING LEGALLY RESPONSIBLE FOR THE SAFETY OF WORKERS UNDER THESE LAWS. IF YOU OR YOUR COMPANY MUST WORK NEAR ENERGIZED OVERHEAD POWER LINES, CALL THE POWER COMPANY FOR THE LINES TO BE DE-ENERGIZED AND/OR MOVED AT YOUR EXPENSE.

7. CONSTRUCTION SHALL COMPLY WITH THE LATEST REVISIONS OF OSHA REGULATIONS AND STATE OF TEXAS LAW CONCERNING TRENCHING AND SHORING. CONTRACTOR SHALL PROVIDE A TRENCH SAFETY SYSTEM TO MEET, AS A MINIMUM, THE REQUIREMENTS OF OSHA SAFETY AND HEALTH REGULATION, PART 1926, SUB-PART P AS PUBLISHED IN THE FEDERAL REGISTER, VOLUME 54, NO. 209, DATED OCTOBER 31, 1989. 8. DETAILS SHOWN DO NOT EXTEND OR INCLUDE DESIGNS OR SYSTEMS PERTAINING TO THE SAFETY OF THE CONTRACTOR OR ITS EMPLOYEES, AGENTS, OR REPRESENTATIVES IN THE PERFORMANCE OF THE WORK. THE CONSTRUCTION CONTRACTOR SHALL PREPARE OR OBTAIN THE APPROPRIATE SAFETY SYSTEMS, INCLUDING THE PLANS AND SPECIFICATIONS REQUIRED BY CHAPTER 756, SUBCHAPTER "C" OF THE TEXAS HEALTH AND SAFETY

9. CONTRACTOR SHALL COVER OPEN EXCAVATIONS WITH ANCHORED STEEL PLATES DURING NON-WORKING HOURS, ALONG EXISTING ROADWAYS AND TRAFFIC AREAS.

10. ADEQUATE DRAINAGE SHALL BE MAINTAINED AT ALL TIMES DURING CONSTRUCTION AND ANY DRAINAGE DITCH OR STRUCTURE DISTURBED DURING CONSTRUCTION SHALL BE RESTORED TO THE SATISFACTION OF THE GOVERNING ENTITY. ALL CONSTRUCTION RUNOFF SHALL COMPLY WITH STORM WATER MANAGEMENT FOR CONSTRUCTION ACTIVITIES AND THE NATIONAL POLLUTANT DISCHARGE ELIMINATION SYSTEM (NPDES) REQUIREMENTS.

11. CONTRACTOR SHALL BE RESPONSIBLE FOR PROVIDING ADEQUATE FLAGMEN, SIGNING, STRIPING AND WARNING DEVICES, ETC., DURING CONSTRUCTION IN ACCORDANCE WITH THE "TEXAS MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES". CONTRACTOR SHALL MAINTAIN AT LEAST ONE LANE OF TRAFFIC IN EACH DIRECTION DURING WORKING HOURS OR PROVIDE ALL-WEATHER DETOURS AROUND CONSTRUCTION SITE, PROVIDE PUBLIC NOTIFICATION, AND USE UNIFORMED POLICE OFFICERS TO CONTROL TRAFFIC.

12. EXISTING PAVEMENTS, CURBS, SIDEWALKS AND DRIVEWAYS DAMAGED OR REMOVED DURING CONSTRUCTION SHALL BE REPLACED TO THE GOVERNING ENTITY'S STANDARDS. ALL ASPHALT AND CONCRETE DRIVEWAYS EXCAVATED DURING CONSTRUCTION SHALL BE BACKFILLED WITH STABILIZED MATERIAL AND RETURNED TO EXISTING CONDITIONS. ALL STATE AND COUNTY HIGHWAY PAVEMENT AND RAILROAD RIGHT-OF-WAYS TO E BORED ACCORDING TO THE RULES, REGULATIONS AND REQUIREMENTS FOR APPROVAL AND ACCEPTANCE BY SAID

13. EXISTING ROADS AND/OR RIGHT-OF-WAYS DISTURBED DURING CONSTRUCTION SHALL BE AS GOOD OR BETTER THAN THE CONDITION PRIOR TO STARTING THE WORK, UPON COMPLETION OF THE PROJECT.

14. AFTER DISTURBED AREAS HAVE BEEN COMPLETED TO THE LINES, GRADES, AND CROSS-SECTIONS SHOWN ON THE PLANS, SEEDING SHALL BE PERFORMED IN ACCORDANCE WITH THE REQUIREMENTS OF THE PLANS AND SPECIFICATIONS TO ESTABLISH ADEQUATE VEGETATION COVERAGE TO ELIMINATE EROSION. IF NO PROVISION FOR PLANTING GRASS IS INCLUDED IN THE PLANS OR SPECIFICATIONS, THE MINIMUM REQUIREMENT FOR THIS ITEM WILL BE IN ACCORDANCE WITH THE TEXAS DEPARTMENT OF TRANSPORTATION STANDARD SPECIFICATIONS FOR "SODDING OR SEEDING FOR EROSION CONTROL."

15. ALL TRENCHES, INCLUDING TRENCHES FOR LEADS AND STUBS UNDER PAVEMENT AND TO A POINT ONE (1) FOOT BACK OF ALL CURBS SHALL BE BACKFILLED WITH CEMENT STABILIZED SAND AS PER SPECIFICATION TO A POINT IMMEDIATELY BELOW THE SUBGRADE. TRENCHES OTHER THAN UNDER PAVEMENT SHALL BE BACKFILLED WITH SUITABLE EARTH MATERIAL IN 6 INCH LAYERS AND MECHANICALLY COMPACTED TO A DENSITY OF NOT LESS THAN 95 PERCENT OF THE MAXIMUM DRY DENSITY AS DETERMINED BY THE STANDARD PROCTOR COMPACTION TEST (ASTM DESIGNATION D-698/AASHTO T99). MOISTURE CONTENT OF BACKFILL SHALL BE IN ACCORDANCE WITH THE REQUIREMENTS OF THE CEMENT STABILIZED SAND SPECIFICATIONS. SEE GOVERNING ENTITY'S STANDARD DETAIL SHEETS FOR BEDDING AND OTHER DESIGN REQUIREMENTS.

16. CONTRACTOR TO REMOVE EXISTING PLUGS AND CONNECT TO EXISTING UTILITY LINES AS INDICATED ON

17. UNLESS OTHERWISE NOTED ON PLANS, WHERE MANHOLES ARE LOCATED WITHIN THE UTILITY EASEMENTS, THE CONTRACTOR SHALL SET RIM ELEVATIONS TWO INCHES ABOVE FINISHED GROUND ELEVATIONS.

18. WHEN TRENCH CONDITION REQUIRES THE USE OF WELL POINTS, THIS IS TO BE REQUESTED BY THE

19. CONTRACTOR SHALL BE RESPONSIBLE FOR CLEANING THE MUD AND/OR DIRT DEPOSITED ON EXISTING PAVEMENT DUE TO HIS CONSTRUCTION ACTIVITY DAILY. ALL EQUIPMENT AND DEBRIS FROM CONSTRUCTION TO BE MOVED AT END OF PROJECT.

STORM WATER QUALITY NOTES:

TO THE TCEQ.

CONTRACTOR AND APPROVED BY THE ENGINEER.

1. COVERAGE IS REQUIRED UNDER THE TPDES GENERAL PERMIT TXR150000 FOR STORM WATER DISCHARGES ASSOCIATED WITH CONSTRUCTION ACTIVITIES. THE CONTRACTOR SHALL BE RESPONSIBLE FOR IMPLEMENTATION, INSPECTION, AND MAINTENANCE OF THE STORM WATER POLLUTION PREVENTION PLAN (SWPPP) IN ACCORDANCE WITH FEDERAL, STATE AND LOCAL REGULATIONS. THE COSTS TO IMPLEMENT, INSPECT, AND MAINTAIN THE SWPPP SHALL BE CONSIDERED INCIDENTAL TO THE SWPPP BID ITEMS.

2. IF THE PROJECT DISTURBS GREATER THAN 5 ACRES, A NOTICE OF INTENT (NOI) SHALL BE SUBMITTED TO THE TEXAS COMMISSION ON ENVIRONMENTAL QUALITY (TCEQ) AT LEAST 7 DAYS PRIOR TO THE START OF ANY EARTH DISTURBING ACTIVITIES. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL TCEQ COMPLIANCE, PLAN IMPLEMENTATION AND MAINTENANCE DURING CONSTRUCTION. WHEN DIRECTED BY THE ENGINEER, THE CONTRACTOR SHALL PROVIDE A COPY OF THE CONTRACTOR'S NOTICE OF INTENT (NOI) AND PROOF THAT IT HAS BEEN SENT TO THE TCEQ.

3. COPIES OF THE CONTACTOR'S NOI AND CONSTRUCTION SITE NOTICE (CSN) SHALL BE POSTED AT THE SITE BY THE CONTRACTOR. COPIES SHALL ALSO BE SUBMITTED TO THE PROJECT OWNER AND ENGINEER. THE CONTRACTOR SHALL LAMINATE AND POST THE TWO NOIS, TWO CSNS AND ANY "SECONDARY OPERATOR" CSNS ON THE PROJECT SITE AT A LOCATION WITH EASY ACCESS TO THE PUBLIC FOR CLEAR VIEWING AND AS APPROVED BY THE ENGINEER. THE COST OF LAMINATION AND POSTING OF THE NOIS & CSNS SHALL BE

CONSIDERED INCIDENTAL TO THE SWPPP BID ITEMS. 4. UPON COMPLETION OF CONSTRUCTION ACTIVITIES AND FINAL STABILIZATION OF THE SITE, AS DEFINED BY THE TPDES GENERAL PERMIT, A NOTICE OF TERMINATION (NOT) IS REQUIRED TO BE SUBMITTED TO THE TEXAS COMMISSION ON ENVIRONMENTAL QUALITY (TCEQ). WHEN DIRECTED BY THE ENGINEER, THE CONTRACTOR SHALL PROVIDE A COPY OF THE CONTRACTOR'S NOTICE OF TERMINATION (NOT) AND PROOF THAT IT HAS BEEN SENT

5. A RAIN GAUGE SHALL BE KEPT ON THE PROJECT SITE OR WITHIN THE IMMEDIATE PROJECT VICINITY. RECORDS OF RAINFALL EVENTS SHALL BE KEPT BY THE CONTRACTOR TO ASSIST WITH DETERMINING IF AN SWPPP SITE INSPECTION IS REQUIRED. THE COSTS FOR THE RAIN GAUGE SHALL BE CONSIDERED INCIDENTAL TO THE SWPPP BID ITEMS.

6. THE SWPPP, INSPECTION & MAINTENANCE REPORTS, CERTIFICATIONS, RAINFALL RECORDS, MAJOR GRADING DATE RECORDS AND TEMPORARY AND PERMANENT STABILIZATION DATE RECORDS SHALL BE KEPT CURRENT BY THE CONTRACTOR AND IN ACCORDANCE WITH STATE AND LOCAL REGULATIONS. COPIES OF THE ALL SWPPP RECORDS SHALL BE KEPT ON-SITE, IF FEASIBLE, UNTIL THE NOTICE OF TERMINATION HAS BEEN SUBMITTED TO THE TEXAS COMMISSION ON ENVIRONMENTAL QUALITY. THE SWPPP RECORDS SHALL BE MADE READILY AVAILABLE TO ENGINEER AND REGULATORY AUTHORITIES UPON AN ON-SITE INSPECTION. THE CONTRACTOR SHALL DELIVER COPIES OF ALL SWPPP RECORDS TO PROJECT OWNER AND ENGINEER AS DIRECTED BY THE FNGINFFR."

SANITARY SEWER CONSTRUCTION NOTES:

1. SANITARY SEWERS SHALL BE CONSTRUCTED IN COMPLIANCE WITH THE LATEST SPECIFICATIONS FOR SEWER CONSTRUCTION, AND TESTED AS SPECIFIED FROM THE LATEST TEST PROCEDURE FOR EITHER LIQUID OR AIR, INCLUDING ALL AMENDMENTS AND REVISIONS THERETO. BACKFILL AND BEDDING FOR SANITARY SEWERS MUST MEET ALL MINIMUM ASPECTS OF ASTM D-2321 AND MUST BE PLACED IN ACCORDANCE WITH THE APPLICABLE ENTITY'S SPECIFICATIONS.

2. ALL SANITARY SEWER MANHOLES SHALL BE STANDARD THE APPLICABLE ENTITY PRE-CAST USING RAM-NECK OR CAST IN PLACE CONCRETE IN ACCORDANCE WITH ASTM C-478. NO BRICK MANHOLES ALLOWED. FOR PVC PIPE, USE MANHOLE WATER STOP GASKET AND CLAMP ASSEMBLY AT MANHOLE CONNECTIONS. SANITARY SEWER MANHOLE RIMS SHALL BE 3 INCHES ABOVE NATURAL GROUND. BACKFILL SHALL BE ADDED AND SLOPED AWAY FROM THE MANHOLE RIM FOR DRAINAGE PURPOSES.

3. MANHOLE CONCRETE BOTTOM FOUNDATION SHALL BE 12" REINFORCED WITH #5 BARS AT 12", ON CENTERS, EACH WAY, WITH A MINIMUM OF 6" EXTRA SLAB LENGTH AROUND THE MANHOLE, IF POURED IN PLACE. APPROVED CHEMICALS SHALL BE USED FOR PATCHING AROUND MANHOLE JOINTS. MORTAR CEMENT WILL NOT

4. SANITARY SEWER PIPE SHALL BE PVC SDR 26 OR PVC SDR 35 (WITH APPROVAL), IN ACCORDANCE WITH ASTM SPECIFICATIONS D-3034, FOR 4" THROUGH 15" AND ASTM F-879 FOR 18" THROUGH 27". MINIMUM SIZE SANITARY SEWER MAIN IS 6". SDR 35 MAY BE USED WHEN DEPTH IS MORE THAN 3 FEET AND LESS

5. SEWER LINES SHALL BE LOCATED ON THE OPPOSITE SIDE OF THE STREET FROM WHERE WATER IS LOCATED. SEWER LINE AND WATER LINE SEPARATION SHALL BE IN ACCORDANCE WITH TEXAS NATURAL RESOURCE CONSERVATION COMMISSION RULES, CHAPTER 317.13 APPENDIX E.

6. NO SEWER PIPE SHALL BE LAID ON AN UNSTABLE FOUNDATION. SELECTED MATERIAL SHALL BE USED AND/OR WET SAND CONSTRUCTION DETAILS, WHICHEVER APPLIES IN THE OPINION OF THE ENGINEER. NO PIPE SHALL BE COVERED WITHOUT APPROVAL OF THE ENGINEER OR HIS REPRESENTATIVE. SANITARY SEWERS CONSTRUCTED IN WET SAND SHALL HAVE A SPECIAL PROCEDURE AND SHALL BE CONSTRUCTED AS PER THE

7. WHEN THE NATURAL GROUND LEVEL AROUND MANHOLE LIES BELOW THE 100 YEAR FLOODPLAIN ELEVATION, THE MANHOLE SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE SEALED AND VENTED MANHOLE DETAIL.

8. A DEFLECTION TEST SHALL BE REQUIRED AFTER THE BACKFILL HAS BEEN IN PLACE A MINIMUM OF 30 DAYS. THIS TEST SHALL BE DONE BY PULLING A HAND LINE WITH AN ATTACHED MANDREL FROM MAN-HOLE TO MANHOLE. THE MANDREL SHALL HAVE AN OUTSIDE DIAMETER THAT IS AT LEAST 95% OF THE ORIGINAL INSIDE DIAMETER OF THE PIPE. MANDREL TO BE MANUFACTURED WITH A MINIMUM OF SEVEN (7) RUNNERS, WITH EACH RUNNER BEING A MINIMUM OF 5 INCHES LONG. ANY PIPE NOT MEETING EST REQUIREMENTS TO BE REMOVED AND REPLACED AT THE CONTRACTOR'S EXPENSE. THE TEST SHALL BE PERFORMED WITHOUT

9. INFILTRATION/EXFILTRATION NOT TO EXCEED 200 GALLONS PER INCH DIAMETER PER MILE OF PIPE FOR 24 HOURS UNDER A MINIMUM OF 2 FEET OF HEAD, OR AN AIR TEST SHALL BE REQUIRED IN ACCORDANCE WITH

10. WHERE A SEWER LINE HAS LESS THAN (2) FEET OF COVER, PROVIDE CEMENT STABILIZED SAND BACKFILL

11. CONTRACTOR SHALL KEEP RECORD OF LOCATION OF ALL STACKS, STUBS, SEWER LEADS, ETC. THE AS-BUILT MYLAR DRAWINGS MUST SHOW THE EXACT LOCATION.

12. IF SANITARY SERVICE LEADS ARE INSTALLED DURING CONSTRUCTION OF MAIN LINE, ALL LEADS TO HAVE A MINIMUM SLOPE OF 0.70% OR GREATER. ALL PVC LEADS TO BE THE SAME MATERIAL AS MAIN LINE. ALL DOUBLE SERVICE LEADS TO HAVE WYE LOCATED ON THE END OF THE LEAD. ALL SINGLE SERVICE LEADS TO BE 4 INCH, AND ALL DOUBLE SERVICE LEADS TO BE 6 INCH.

13. THE INSTALLATION OF ALL SANITARY SEWER LINES SHALL EXTEND ALONG THE ENTIRE LENGTH OF THE PROPERTY TO BE SERVED. SANITARY SEWER LINES THAT DEAD END SHALL EXTEND TO THE PROJECT LIMITS FOR FUTURE EXTENSIONS, WITH DEPTHS BASED ON ENTIRE SERVICE AREA.

PAVING NOTES:

ASTM C-828.

1. IF PROPOSED SEMI-RIGID BASE WITH 2 INCH TYPE "D" HOT MIX ASPHALTIC CONCRETE SURFACING, FOR URBAN ESTATES ONLY, SEMI-RIGID BASE MAY BE 7 INCH CEMENT STABILIZED SHELL, 8 INCH CRUSHED LIMESTONE, OR 6 INCH HOT MIX ASPHALTIC CONCRETE.

2. EXPOSE 15 INCHES OF REINFORCING STEEL AT ALL PROPOSED SAWED JOINTS. IF NO REINFORCING STEEL EXISTS, USE HORIZONTAL DOWELS PER NOTE #4.

3. REQUIRE A ONE (1) INCH REDWOOD EXPANSION BOARD OR PRE-MOLDED NON-EXTRUDING JOINT BETWEEN

SIDEWALK AND BACK OF CURB. 4. HORIZONTAL DOWELS SHALL BE NO. 6 BARS, 24 INCHES LONG, DRILLED AND EMBEDDED 8 INCHES INTO THE CENTER OF THE EXISTING SLAB WITH "PO ROC" OR EQUAL. DOWELS SHALL BE 24 INCHES CENTER TO

CENTER UNLESS OTHERWISE SPECIFIED. 5. WHEN PROPOSED PAVEMENT ENDS AT A CONSTRUCTION JOINT LEAVE 15 INCHES OF REINFORCING STEEL EXPOSED BEYOND PAVEMENT, COAT WITH ASPHALT, AND WRAP WITH BURLAP FOR FUTURE PAVEMENT TIE-IN.

AT EXPANSION JOINTS, EXTEND DOWELS 5 INCHES; COAT AND WRAP SAME AS CONSTRUCTION JOINTS.

6. WHEREVER A SIDEWALK IS REQUIRED BY GOVERNING ENTITY'S ORDINANCE, PROVIDE WHEELCHAIR RAMP AND/OR SIDEWALKS IN ACCORDANCE WITH THE "TEXAS DEPARTMENT OF TRANSPORTATION STANDARD WHEELCHAIR RAMP AND SIDEWALK DETAILS".

7. ADJUST EXISTING MANHOLE FRAMES AND COVERS TO FIT NEW GRADE.

8. ADJUST EXISTING WATER VALVE BOXES TO NEW PAVING GRADE. REPLACE ALL MISSING OR DAMAGED VALVE BOXES AND COVERS.

9. PLACE WHITE OR YELLOW PLASTIC MARKER OR PAINT AS SHOWN BY THE UNIFORM TRAFFIC MANUAL FOR PAVEMENT MARKINGS.

10. PROVIDE A CONCRETE PAVING HEADER AT THE END OF THE PAVEMENT.

11. T. C. INDICATES TOP OF CURB ELEVATION AND T. P. INDICATES TOP OF PAVEMENT ELEVATION. 12. CURB RADII AT STREET INTERSECTIONS TO BE 24.50 FEET TO BACK OF CURB WITH A MINIMUM OF ONE (1) PERCENT GRADE UNLESS OTHERWISE NOTED.

13. GUIDELINES SET FORTH IN THE "TEXAS MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES" WILL BE OBSERVED.

14. TRANSVERSE EXPANSION JOINTS SHALL BE INSTALLED AT ALL RADIUS RETURNS AND AT A MAXIMUM SPACING OF 60 FOOT INTERVALS.

15. CONTRACTOR WILL USE CONTINUOUS LONGITUDINAL REINFORCING BARS IN CURBS AS SHOWN ON DETAILS PROVIDED IN CONSTRUCTION DRAWINGS.

16. A CONCRETE MIX DESIGN BY THE CERTIFIED LAB SHALL BE SUBMITTED TO AND APPROVED BY THE GOVERNING ENTITY'S ENGINEER BEFORE ANY CONCRETE IS POURED.

17. CONSTRUCTION OF ITEMS THAT ARE NOT SPECIFICALLY ADDRESSED BY CITY OF BRYAN REQUIREMENTS TO BE IN ACCORDANCE WITH THE TEXAS HIGHWAY DEPARTMENT STANDARD SPECIFICATIONS (LATEST REVISION). 18. RIGHT-OF-WAY SHALL BE SLOPED FROM THE PROPERTY TO THE TOP OF CURB AND HYDROMULCHED OR SODDED BEFORE FINAL ACCEPTANCE BY THE GOVERNING ENTITY TO CONTROL EROSION INTO THE STREET AND

19. MEMBRANE CURING TYPE 2, WHITE PIGMENTED, SHALL BE USED FOR CURING ALL CONCRETE SURFACES IMMEDIATELY AFTER FINISHING OF SURFACES AND SHALL BE IN ACCORDANCE WITH THE TEXAS HIGHWAY DEPARTMENT STANDARD SPECIFICATIONS ITEM #526.

20. ALL FIRST STAGE INLET CONSTRUCTION SHALL BE PROTECTED WITH 3 INCH THICK BOARDS AT ALL TIMES. 21. ALL SUBGRADE AND EMBANKMENT AREAS SHALL BE STRIPPED OF ALL ORGANIC AND UNSUITABLE MATERIAL BEFORE STABILIZATION OR FILLING IS BEGUN. MATERIAL USED FOR FILL SHALL BE CERTIFIED BY A LAB TO HAVE A PLASTICITY INDEX BETWEEN 10 AND 20.

22. FORMS SHALL BE SET TO THE PROPER GRADE AND PROPERLY SUPPORTED SO THAT NO DISPLACEMENT OCCURS WITH THE PAVING ACTIVITIES. ALL CONCRETE SHALL BE VIBRATED BY MECHANICAL MEANS TO INSURE PROPER COMPACTION AND NO HONEY COMBS.

23. CONCRETE SHALL NOT BE PLACED WHEN THE TEMPERATURE IS BELOW 40° F. AND FALLING, BUT MAY BE

PLACED WHEN TEMPERATURE IS ABOVE 35° F. AND RISING. THE TEMPERATURE SHALL BE TAKEN IN THE SHADE AND AWAY FROM ARTIFICIAL HEAT. 24. THE CONTRACTOR SHALL ERECT AND MAINTAIN BARRICADES TO ADEQUATELY PROTECT THE PAVEMENT. THE

CONTRACTOR SHALL HAVE PERSONNEL ON SITE UNTIL THE PAVEMENT HAS REACHED SUFFICIENT STRENGTH AS NOT TO BE DAMAGED BY ANIMALS OR FOOT TRAFFIC. 25. JOINTS SHALL BE CLEANED OF ALL SCALE, DIRT, DUST, CURING COMPOUND, AND CONCRETE TO THE WIDTH

AND DEPTH OF THE JOINT AND SHALL BE DRY BEFORE SEALING IS PERFORMED.

26. ALL IMPROVEMENTS SHALL BE CONSTRUCTED AND TESTED PER CITY OF BRYAN REQUIREMENTS.

STORM SEWER NOTES:

1. STORM SEWER AND LEADS SHALL BE REINFORCED CONCRETE PIPE, ASTM C-76, CLASS III, WITH O-RING RUBBER GASKET JOINTS, AND SHALL BE INSTALLED, BEDDED AND BACKFILLED IN ACCORDANCE WITH THE GOVERNING ENTITIE'S STANDARDS AND SPECIFICATIONS

NOTE: HDPE PIPE MAY BE USED PROVIDED THAT IT IS BACKFILLED WITH CEMENT STABILIZED SAND (2 SACKS CEMENT/TON), OR OTHER BACKFILL MATERIALS THAT HAVE BEEN APPROVED BY THE GOVERNING ENTITY. SEE

2. ALL PROPOSED PIPE STUB OUTS FROM MANHOLES OR INLETS ARE TO BE PLUGGED WITH 8 INCH BRICK WALLS UNLESS OTHERWISE NOTED.

3. ALL BOX CULVERTS INSTALLED SHALL BE PLACED ON A MINIMUM OF 6 INCHES OF CEMENT STABILIZED SAND (CEMENT STABILIZED SAND SHALL BE 13 SACK CEMENT PER TON). FOR INSTALLATION OF PRE-CASE CONCRETE BOX CULVERTS IN POOR SAIL CONDITIONS, A 7 INCH REINFORCED CONCRETE SLAB SHALL BE INSTALLED. FOR INSTALLATION OF MONOLITHIC REINFORCED CONCRETE BOX CULVERTS IN POOR SOIL CONDITIONS, A 4 INCH THICK CLASS "C" CONCRETE SEAL SLAB SHALL BE INSTALLED, PRIOR TO CONSTRUCTION

4. STORM SEWER MANHOLES SHALL BE STANDARD PRE—CAST, UNLESS OTHERWISE NOTED.

5. ALL INLETS TO BE TO THE DETAIL SPECIFICATIONS SHOWN IN THE PLANS OR APPROVED EQUAL OR UNLESS OTHERWISE STATED ON PLANS. INLETS TO BE STANDARD DEPTH UNLESS OTHERWISE NOTED.

6. ALL STORM SEWER LEADS SHALL BE 18 INCH MINIMUM UNLESS OTHERWISE INDICATED. GRADE DROP ON LEADS BETWEEN INLETS TO BE A MINIMUM OF 0.20 FOOT. GRADE DROP BETWEEN INLET AND MANHOLES TO BE 0.20 FOOT UNLESS OTHERWISE SHOWN. WHEN MANHOLE FRAME AND COVER IS REQUIRED, USE EAST JORDAN 24" FRAME AND COVER (OR EQUAL).

9. FOR ADJUSTMENT OF MANHOLE LIDS USE STANDARD CONCRETE RINGS.

10. CONCRETE USED FOR ALL POURED-IN-PLACE MANHOLES, INLETS, WINGWALLS, HEADWALLS AND OTHER APPURTENANCES TO BE CLASS "A" CONCRETE WITH 3,000 P.S.I. STRENGTH AT 28 DAYS.

11. ALL EXPOSED CORNERS TO BE CHAMFERED 3/4".

12. OTHER BACKFILL MATERIALS MAY BE USED BASED ON THE GEOTECHNICAL REPORT OR PER HDPE SPECIFICATIONS. BACKFILL MUST BE USED WITH APPROPRIATE COMPACTION.

13. SEE MANUFACTURERS SPECIFICATIONS FOR THE USE OF HIGH DENSITY POLYETHYLENE PIPE FOR STORM DRAINS FOR SPECIFIC TECHNICAL INFORMATION.

WATER CONSTRUCTION NOTES:

1. CONTRACTOR SHALL PROVIDE ADEQUATE THRUST BLOCKING TO WITHSTAND TEST PRESSURE AS SPECIFIED IN THE APPLICABLE ENTITY STANDARD DRAWINGS AND REQUIREMENTS FOR WATER MAIN CONSTRUCTION AND

2. PRIOR TO INSTALLATION OF WATER METER, WATER METER LEAD OR UNMETERED FIRE SPRINKLER LINE, THE CONTRACTOR SHALL CONTACT THE PERMIT DIVISION.

3. PRIOR TO WATER MAIN CONSTRUCTION, THE CONTRACTOR SHALL CONTACT THE GOVERNING ENTITY'S ENGINEER AND COMPLY WITH ALL REQUIREMENTS NECESSARY FOR THE ISSUANCE OF A WORK ORDER FOR THE WATER MAIN CONSTRUCTION.

4. SEPARATION DISTANCES FOR ALL WATER MAIN AND SANITARY SEWER MAIN CONSTRUCTION SHALL BE GOVERNED BY THE "TEXAS NATURAL RESOURCES CONSERVATION COMMISSION RULES AND REGULATIONS FOR DESIGN CRITERIA FOR SEWERAGE SYSTEMS", SECTION 317.20, LATEST PRINTING. REFER TO THE THE APPLICABLE ENTITY DESIGN MANUAL WATER MAIN DESIGN REQUIREMENTS

5. TWELVE-INCH (12") AND SMALLER MAINS SHALL HAVE A MINIMUM COVER OF FOUR FEET (4') FROM THE TOP OF THE CURB OR FIVE FEET (5') FROM THE MEAN ELEVATION OF THE BOTTOM OF THE NEARBY DITCH AND NEARBY RIGHT-OF-WAY ELEVATION FOR OPEN DITCH SECTIONS.

6. MAINS LARGER THAN TWELVE-INCHES (12") SHALL HAVE A MINIMUM COVER OF FIVE FEET (5') FROM THE TOP OF THE CURB OR SIX FEET (6') FROM THE MEAN ELEVATION FOR OPEN DITCH SECTIONS. 7. ALL WATER MAINS SHALL BE HYDROSTATICALLY TESTED BEFORE BACTERIOLOGICAL TESTING IN ACCORDANCE

WITH AWWA STANDARD C-600. 8. ALL WATER PIPING SHALL BE DISINFECTED AND BACTERIOLOGICALLY TESTED PRIOR TO USE IN ACCORDANCE WITH AWWA STANDARD C-601.

9. ALL WATER MAINS 4" THROUGH 12" SHALL BE C-900 (SDR-18). ALL WATER MAINS 14" THROUGH 36" SHALL BE C-905 (SDR-18). 10. PRIOR TO BACKFILLING OF ALL UNDERGROUND WATER LINES, INSTALL A CONTINUOUS #14 COPPER TRACER

11. THE INSTALLATION OF ALL WATER LINES SHALL EXTEND ALONG THE ENTIRE LENGTH OF THE PROPERTY TO

BE SERVED. WATER LINES THAT DEAD END SHALL EXTEND TO THE PROJECT LIMITS FOR FUTURE EXTENSIONS.

WIRE, LOCATED DIRECTLY OVER BURIED LINES AND ACCESSIBLE AT EACH VALVE STACK.

---- EX SANITARY, SAN ----- EX WATERLINE, WL EX STORM SEWER, STM →···→··· EX DRAINAGE PATH, FL ----v----v EX HIGH BANK, HB ---- EX EASEMENT, ESMT --- --- EX BUILDING LINE, BL P EX OVERHEAD POWER, P ----- UG ----- EX UNDERGROUND POWER, UG —— FO —— EX FIBER, FO ----- T ----- EX TELEPHONE, T — G — EX GAS LINE, G — X — EX FENCE, FNC PROJECT BOUNDARY LINE, BNDY PROP SANITARY, SAN PROP WATERLINE, WL PROP STORM SEWER, STM PROP DRAINAGE PATH, FL PROP HIGH BANK, HB — — PROP EASEMENT, ESMT ------ PROP BUILDING LINE, BL PROP OVERHEAD POWER, P — UG — PROP UNDERGND POWER, UG — FO —— PROP FIBER, FO **T** PROP TELEPHONE, ——— G ———— PROP GAS LINE, G — X — PROP FENCE, FNC PROP PAVEMENT, PVMT — — — — — PROP BACK OF CURB, BC

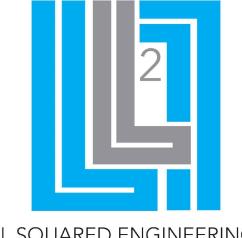
EX ADJOINER LINE, ADJ

BENCHMARK:

FLOODPLAIN NOTE:

THE BEARINGS SHOWN HEREON ARE BASED ON NAD.83, TEXAS CENTRAL ZONE.

THE PROPERTY IS LOCATED WITHIN AN AREA HAVING A ZONE DESIGNATION X BY THE FEDERAL EMERGENCY MANAGEMENT AGENCY (FEMA), ON FLOOD INSURANCE RATE MAP NO. 48041C0215F, WITH A DATE OF IDENTIFICATION OF APRIL 2, 2014, IN BRAZOS COUNTY, STATE OF TEXAS, IN WHICH THE PROPERTY IS SITUATED.



SQUARED ENGINEERING UNICIPAL COMMERCIAL RESIDENTIA

WWW.L2ENGINEERING.COM 3307 W DAVIS STREET #100 CONROE, TEXAS 77304 OFFICE: 936-647-0420 21123 EVA STREET #200 MONTGOMERY, TEXAS 77356

CLIENT INFORMATION JUSTIN WALTON 713-446-4083 JW@BUILD-OLOGY.NET

2539 E VILLA MARIA ROAD BRYAN, TEXAS 77803

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DRAWING ISSUE # DATE BY \* COMMENT 1 9/27/22 JTW FOR PERMIT

DRAWING INFORMATION

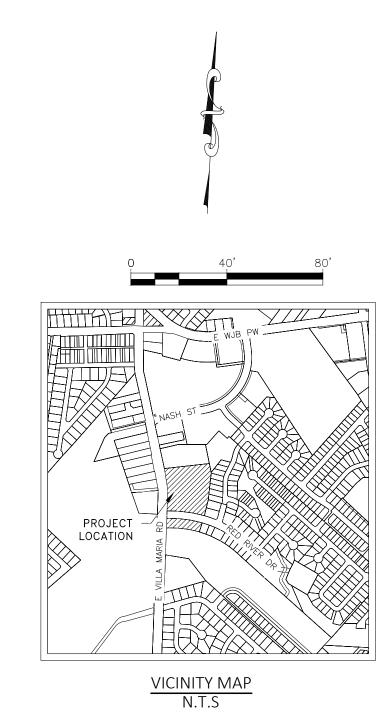
10766 TDLR

)RAWN BAS EIT SHEET JONATHAN T. WHIT

01/10/2023



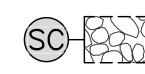




#### SITE SPECIFIC NOTES:

- 1. The owner and developer of the property is Buildology, LLC. The Subject property is The Mods Subdivision, Block 1, Lot 1 & 2.
- 2. The proposed buildings are (13) Two-Story Type, VB with sprinklers totaling 38,559 SF with 4 Units per Building. The riser room will be required to have an exterior door along with FDC signs and FDC Knox Caps.
- 3. Proposed use and improvements: Apartments with parking, utilities & landscaping.
- 4. The subject property is zoned (MF) Multi-Family District per Ordinance No. 2557 approved by Bryan City Council on 5/17/22.
- 5. This lot is within the 100-yr floodplain according to the DFIRM for Brazos County, Texas and effective April 2, 2014.
- 6. The developed area for this project is 4.56 acres (198,634 sf).
- 7. Proposed Signage must comply with Sec. 98-43 and must be permitted separately.
- 8. 12'x12' dumpster pad w/ 10' apron 8" thick reinforced concrete w/ #5 rebar @ 12" O.C.E.W Prior to placement of concrete, contractor shall contact Solid Waste at 979-209-5900 for
- 9. Where electric facilities are installed, BTU has the right to install, operate, relocate, construct, reconstruct, add too, maintain, inspect, patrol, enlarge, repair, remove and replace said facilities upon, over, under, and across the property included in the PUE, and the right of ingress and egress on property adjacent to the PUE to access electric facilities.
- 10. No current Parking Lot Lighting is proposed. If lighting is to be installed, it shall be placed 2' offset from all storm sewer.

#### SWPP PLAN LEGEND:

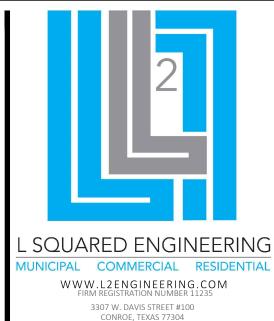


STABILIZED CONSTRUCTION



CONCRETE TRUCK WASHOUT

Curve Table													
Curve #	Length	Radius	Delta	Chord Direction	Chord Length								
C1	447.54	2234.31	11.48	N3° 48' 33"W	446.79								
C2	307.63	1228.55	14.35	N81° 06' 42"W	306.83								
С3	288.16	1148.56	14.37	S81° 05' 52"E	287.40								
C4	261.90	1043.38	14.38	N81° 05' 36"W	261.21								



CONROE, TEXAS 77304 OFFICE: 936-647-0420 21123 EVA STREET #200 MONTGOMERY, TEXAS 77356

**CLIENT INFORMATION** JUSTIN WALTON 713-446-4083 JW@BUILD-OLOGY.NET

PROJECT ADDRESS 2539 E VILLA MARIA ROAD

BRYAN, TEXAS 77803

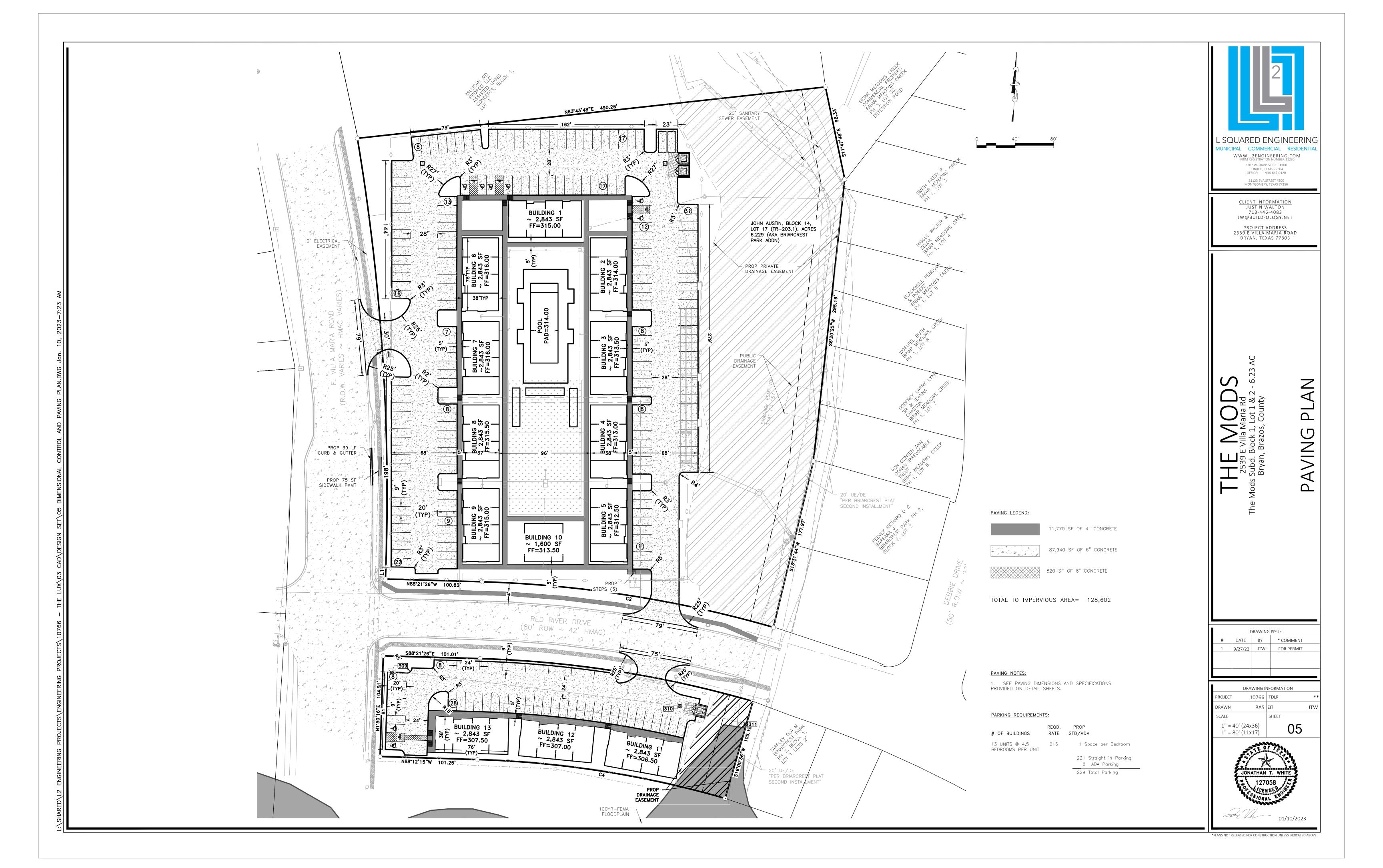
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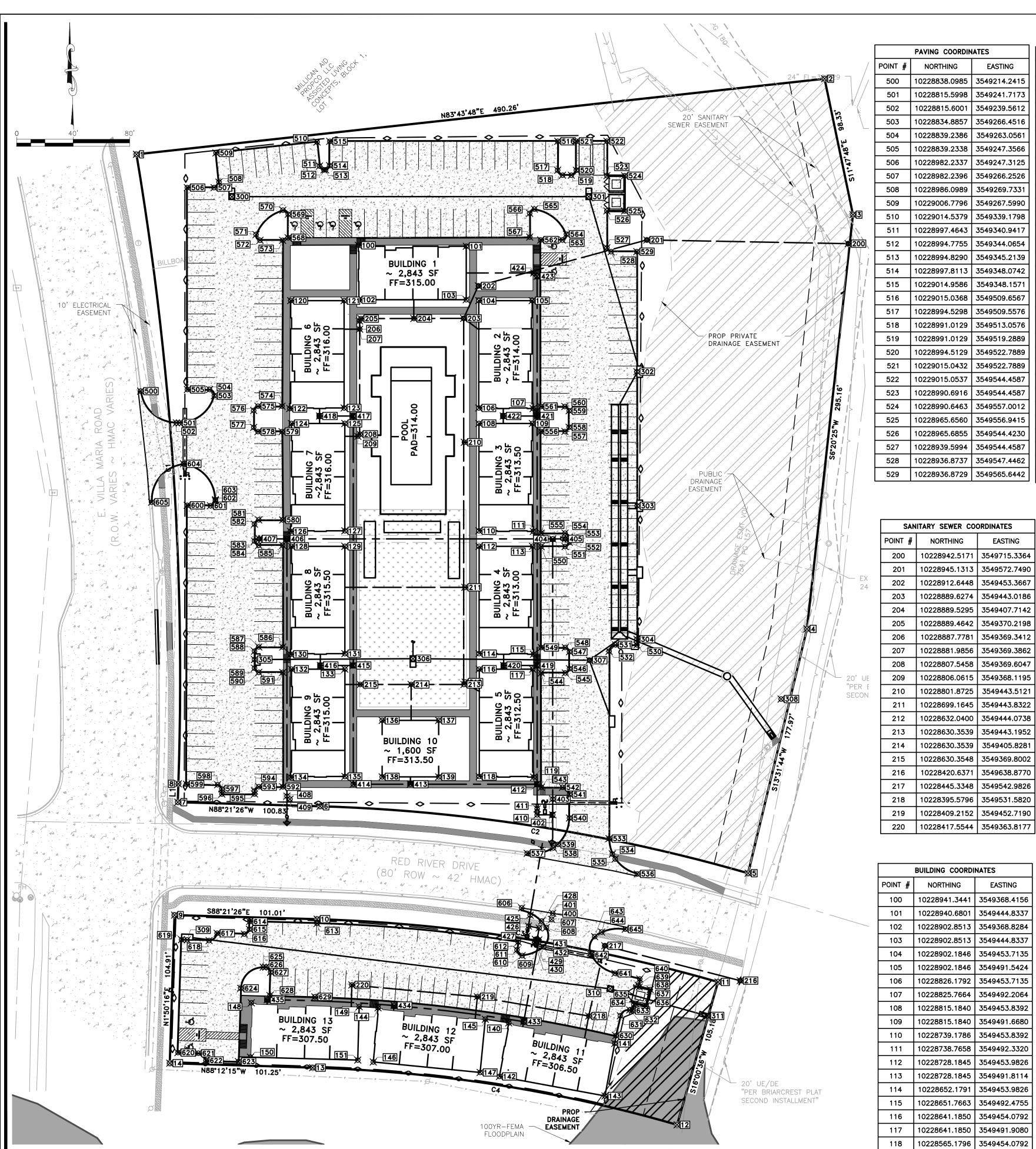
## DRAWING ISSUE # DATE BY \* COMMENT

1 9/27/22 JTW FOR PERMIT

I		
WING II	NFORMATION	
10766	TDLR	**
BAS	EIT	JTW
	SHEET	
(36) (17)	04	
	***	
ATHAN	T. WHITE	
127 معرور کی		
SIOM I	I E NO	
	10766 BAS (36) (17)	BAS EIT SHEET (36) (17) 04

\*PLANS NOT RELEASED FOR CONSTRUCTION UNLESS INDICATED ABOVE





	NATES	PAVING COORDIN		AVING COORDINATES PAVING COORDINATES										
PO	EASTING	NORTHING	POINT #	EASTING	NORTHING	POINT #	EASTING	NORTHING	INT #					
	3549514.6921	10228827.1872	560	3549565.7272	10228658.3743	530	3549214.2415	10228838.0985	500					
	3549497.6841	10228827.1872	561	3549550.5345	10228658.3743	531	3549241.7173	10228815.5998	501					
	3549497.5775	10228945.1351	562	3549546.0345	10228653.8743	532	3549239.5612	10228815.6001	502					
	3549512.9328	10228945.1351	563	3549545.9300	10228521.2014	533	3549266.4516	10228834.8857	503					
	3549515.7346	10228949.2077	564	3549548.3402	10228510.2990	534	3549263.0561	10228839.2386	504					
	3549493.1597	10228966.9679	565	3549550.3364	10228506.7922	535	3549247.3566	10228839.2338	505					
	3549489.7767	10228963.9888	566	3549565.9883	10228496.2084	536	3549247.3125	10228982.2337	506					
	3549489.7974	10228947.1976	567	3549488.0125	10228510.7548	537	3549266.2526	10228982.2396	507					
	3549318.7276	10228946.6994	568	3549506.5158	10228514.5797	538	3549269.7331	10228986.0989	508					
	3549318.7656	10228963.3260	569	3549509.6971	10228517.0618	539	3549267.5990	10229006.7796	509					
	3549314.9903	10228966.2309	570	3549517.9694	10228535.7924	540	3549339.1798	10229014.5379	510					
	3549295.9920	10228948.9546	571	3549518.0153	10228552.5110	541	3549340.9417	10228997.4643	511					
	3549298.8023	10228944.9082	572	3549513.0153	10228557.5248	542	3549344.0654	10228994.7755	512					
	3549314.8247	10228944.9010	573	3549498.0814	10228557.5248	543	3549345.2139	10228994.8290	513					
	3549314.3525	10228827.4012	574	3549497.9761	10228638.5248	544	3549348.0742	10228997.8113	514					
	3549297.3402	10228827.4089	575	3549514.9722	10228638.5248	545	3549348.1571	10229014.9586	515					
	3549294.3388	10228824.4103	576	3549517.9722	10228641.5287	546	3549509.6567	10229015.0368	516					
	3549294.3334	10228812.4103	577	3549517.9566	10228653.5287	547	3549509.5576	10228994.5298	517					
	3549297.3320	10228809.4089	578	3549514.9566	10228656.5248	548	3549513.0576	10228991.0129	518					
	3549314.3562	10228809.4012	579	3549497.9527	10228656.5248	549	3549519.2889	10228991.0129	519					
	3549314.8627	10228746.9010	580	3549497.8357	10228728.0248	550	3549522.7889	10228994.5129	520					
	3549297.8667	10228746.9087	581	3549514.8318	10228728.0248	551	3549522.7889	10229015.0432	521					
	3549294.8653	10228743.9100	582	3549517.8318	10228731.0287	552	3549544.4587	10229015.0537	522					
	3549294.8599	10228731.9101	583	3549517.8162	10228734.5287	553	3549544.4587	10228990.6916	523					
	3549297.8585	10228728.9087	584	3549514.8162	10228737.5248	554	3549557.0012	10228990.6463	524					
	3549314.8661	10228728.9010	585	3549497.8123	10228737.5248	555	3549556.9415	10228965.6560	25					
	3549314.8776	10228656.9010	586	3549497.7070	10228809.5248	556	3549544.4230	10228965.6855	26					
	3549297.8876	10228656.9087	587	3549514.6995	10228809.5244	557	3549544.4587	10228939.5994	527					
	3549294.8862	10228653.9080	588	3549517.6996	10228812.5263	558	3549547.4462	10228936.8737	528					
	3549294.8890	10228641.9080	589	3549517.6921	10228824.1891	559	3549565.6442	10228936.8729	529					

WATERLINE COORDINATES

400 | 10228464.0453 | 3549505.3220

406 | 10228733.4555 | 3549317.8653 |

408 | 10228551.2717 | 3549317.8653

410

414

10228468.3275 | 3549506.0646

10228538.0141 3549506.0646

10228549.1498 | 3549506.0646

10228733.4348 3549506.0646

| 10228733.4766 | 3549515.0709 |

10228733.4576 3549298.1723

10228549.1498 | 3549319.9853

10228538.0141 3549495.0820

10228542.2776 3549495.0538

10228559.7152 3549495.0802

10228559.7152 3549405.3016

10228559.7152 3549364.8538

10228643.8314 | 3549341.5223

415 | 10228643.7859 | 3549364.8538

417 | 10228820.4762 | 3549364.8538

**BUILDING COORDINATES** 

120 | 10228902.1849 | 3549320.5164

121 | 10228902.1849 | 3549358.3452

122 | 10228825.7667 | 3549319.8529

124 | 10228815.1852 | 3549321.0282

125 | 10228815.1852 | 3549358.8571

126 | 10228738.7671 | 3549320.3642

127 | 10228739.1799 | 3549358.8571

128 | 10228728.1856 | 3549321.0449

129 | 10228728.1856 | 3549358.8737

131 | 10228652.1802 | 3549358.8737

133 | 10228641.1859 | 3549358.9054

134 | 10228564.7678 | 3549320.4126

135 | 10228565.1806 | 3549358.9054

136 | 10228604.9290 | 3549385.3017

138 | 10228564.9292 | 3549385.3017

139 | 10228564.9292 | 3549425.3016

10228651.7674 3549320.3809

10228641.1859 3549321.0766

10228604.9290 3549425.3016

10228826.1795 | 3549358.3452

EASTING

POINT # NORTHING

123

132

**EASTING** 

NORTHING

| 10228912.6448 | 3549453.3667

10228889.6274 3549443.0186

10228889.5295 3549407.7142

10228881.9856 3549369.3862

10228806.0615 | 3549368.1195

10228699.1645 | 3549443.8322

10228417.5544 | 3549363.8177

10228941.3441 3549368.4156

10228940.6801 | 3549444.8337

10228902.8513 | 3549368.8284

10228902.8513 | 3549444.8337

10228902.1846 3549453.7135

10228902.1846 | 3549491.5424

10228826.1792 | 3549453.7135 |

10228825.7664 3549492.2064

10228815.1840 | 3549453.8392 |

10228815.1840 | 3549491.6680

10228739.1786 | 3549453.8392

10228738.7658 3549492.3320

10228728.1845 | 3549453.9826

10228652.1791 | 3549453.9826

10228641.1850 | 3549491.9080

10228565.1796 3549454.0792

10228564.7668 | 3549492.5720

**EASTING** 

**BUILDING COORDINATES** 

NORTHING

WATERLINE COORDINATES

10228820.4762 | 3549340.9938

| 10228643.8314 | 3549495.0731

10228643.8304 3549471.3341

10228820.4759 | 3549495.0573

10228820.4759 | 3549471.0650

10228466.9741 3549490.1187

10228452.1177 | 3549487.8663 |

| 10228457.8783 | 3549496.4780

| 10228447.9127 | 3549493.3896 |

10228449.4633 | 3549496.2389 |

10228447.5341 | 3549495.8241

| 10228403.0126 | 3549393.7424 |

433 | 10228391.5590 | 3549485.5993 |

435 | 10228407.1145 | 3549303.9258

BUILDING COORDINATES

140 | 10228390.7030 | 3549474.9453

141 | 10228376.1721 | 3549549.9722

142 | 10228352.7753 | 3549468.3608 |

146 | 10228363.0264 | 3549379.0268 |

149 | 10228402.1923 | 3549369.0268 |

150 | 10228366.4206 | 3549292.0319

151 | 10228364.3772 | 3549368.0098

10228338.9723 | 3549543.1023

10228401.3757 | 3549382.3727

10228393.2581 3549458.3608

10228355.6098 | 3549454.6695

10228404.9106 | 3549292.6541

POINT # | NORTHING |

423 | 10228919.4176 | 3549495.0498

424 | 10228921.7657 | 3549492.7017 |

426 | 10228459.2127 | 3549488.9420 |

10228449.8781

EASTING

3549493.7321

EASTING

NORTHING

421

422

428

430

	10228786.6027	3549244.5750		634	10228403.7069	3549563.516							
605	10228759.4087	3549222.0006		635	10228404.4523	3549564.722							
606	10228472.1396	3549484.0828		636	10228402.6353	3549572.350							
607	10228462.7943	3549498.1895		637	10228414.3040	3549575.129							
608	10228458.2321	3549501.5514		638	10228416.1447	3549567.40°							
609	10228444.5778	3549499.6094		639	10228417.3464	3549566.660							
610	10228438.3770	3549484.4541		640	10228421.7818	3549567.75							
611	10228442.4109	3549480.7809		641	10228425.8094	3549550.176							
612	10228447.9177	3549481.6417		642	10228438.7717	3549533.164							
613	10228461.1270	3549339.1456		643	10228451.7816	3549535.758							
614	10228462.4946	3549291.4588		644	10228454.9978	3549540.74							
615	10228456.9969	3549291.3011	Ī	645	10228457.0683	3549557.43							
616	10228453.5987	3549287.7022	•										
617	10228454.1528	3549268.3812											
618	10228448.9986	3549262.7156	549262.7156										
619	10228449.6243	3549243.2143											
			,										
			•										
	BOUNDARY COOR	DINATES		S1	ORM SEWER COO	RDINATES							
POINT #	. 1	DINATES EASTING	7	ST POINT #	ORM SEWER COO	RDINATES EASTING							
POINT #	. 1	EASTING	5		1	<u> </u>							
•	NORTHING	EASTING 3549211.2950	-	POINT #	NORTHING	EASTING							
1	NORTHING 10229005.7948	EASTING 3549211.2950 3549698.6182	2	POINT #	NORTHING 10228975.7829	EASTING 3549278.83							
1 2	NORTHING 10229005.7948 10229059.3289	EASTING 3549211.2950 3549698.6182 3549718.7206	<u>2</u>	POINT # 300 301	NORTHING 10228975.7829 10228975.8269	EASTING 3549278.83 3549531.62							
1 2 3	NORTHING 10229005.7948 10229059.3289 10228963.0755	EASTING  3549211.2950  3549698.6182  3549718.7206  3549686.1245	2:   35   15	POINT # 300 301 302	NORTHING 10228975.7829 10228975.8269 10228851.8570	EASTING 3549278.83 3549531.62 3549565.67							
1 2 3 4	NORTHING 10229005.7948 10229059.3289 10228963.0755 10228669.7206	EASTING  3549211.2950  3549698.6182  3549718.7206  3549686.1245  3549644.4915	3	POINT # 300 301 302 303	NORTHING 10228975.7829 10228975.8269 10228851.8570 10228756.8742	EASTING 3549278.83 3549531.62 3549565.67 3549565.87							

PAVING COORDINATES

620 | 10228369.6655 | 3549240.6487

622 | 10228364.0270 | 3549260.4666

623 | 10228363.2887 | 3549283.4774

624 | 10228415.2047 | 3549285.1272

625 | 10228430.2060 | 3549301.0638 |

626 | 10228430.1532 | 3549302.9047

627 | 10228426.5543 | 3549306.3029 |

628 | 10228410.0611 | 3549305.8299

629 | 10228409.1493 | 3549337.6217

630 | 10228381.6433 | 3549550.7241

631 | 10228395.7597 | 3549553.9880

632 | 10228399.8736 | 3549560.6098

633 | 10228399.4292 | 3549562.4857

10228369.2001 | 3549255.1526

EASTING

POINT # | NORTHING

PAVING COORDINATES

0228638.9087 3549297.8876

10228638.9010 | 3549314.8897

10228557.9010 | 3549314.9147

10228557.9087 | 3549297.9085 |

10228554.9100 | 3549294.9072

10228552.8078 | 3549294.9062 |

10228552.7649 | 3549271.7799

0228559.3466 3549268.7792

10228559.7359 3549246.6905

10228757.2271 3549247.2129

10228756.9373 | 3549263.6490

10228760.4364 | 3549267.2107

10228761.2610 | 3549267.2108 |

EASTING

3549271.7260

NORTHING

0228556.3916

10228546.9886	3549240.5585	306	10228648.5973	3549407.154
10228559.9915	3549240.9768	307	10228647.5286	3549532.054
10228467.0214	3549238.2650	308	10228620.0312	3549667.838
10228464.1257	3549339.2335	309	10228449.4961	3549247.212
10228419.6509	3549623.1714	310	10228414.8250	3549546.847
10228318.5505	3549594.1622	311	10228395.6533	3549615.735
10228358.9924	3549336.1020			
10228362.1654	3549234.9006			

1. ALL POINTS SHOWN ON THE PAVEMENT ARE TO THE BACK OF CURB UNLESS OTHERWISE NOTED.

The M		

SQUARED ENGINEERING

WWW.L2ENGINEERING.COM

3307 W. DAVIS STREET #100 CONROE, TEXAS 77304 OFFICE: 936-647-0420

21123 EVA STREET #200 MONTGOMERY, TEXAS 77356

CLIENT INFORMATION JUSTIN WALTON

713-446-4083 JW@BUILD-OLOGY.NET

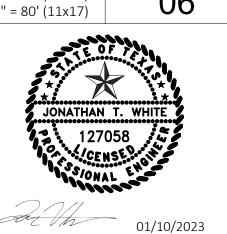
PROJECT ADDRESS 2539 E VILLA MARIA ROAD BRYAN, TEXAS 77803

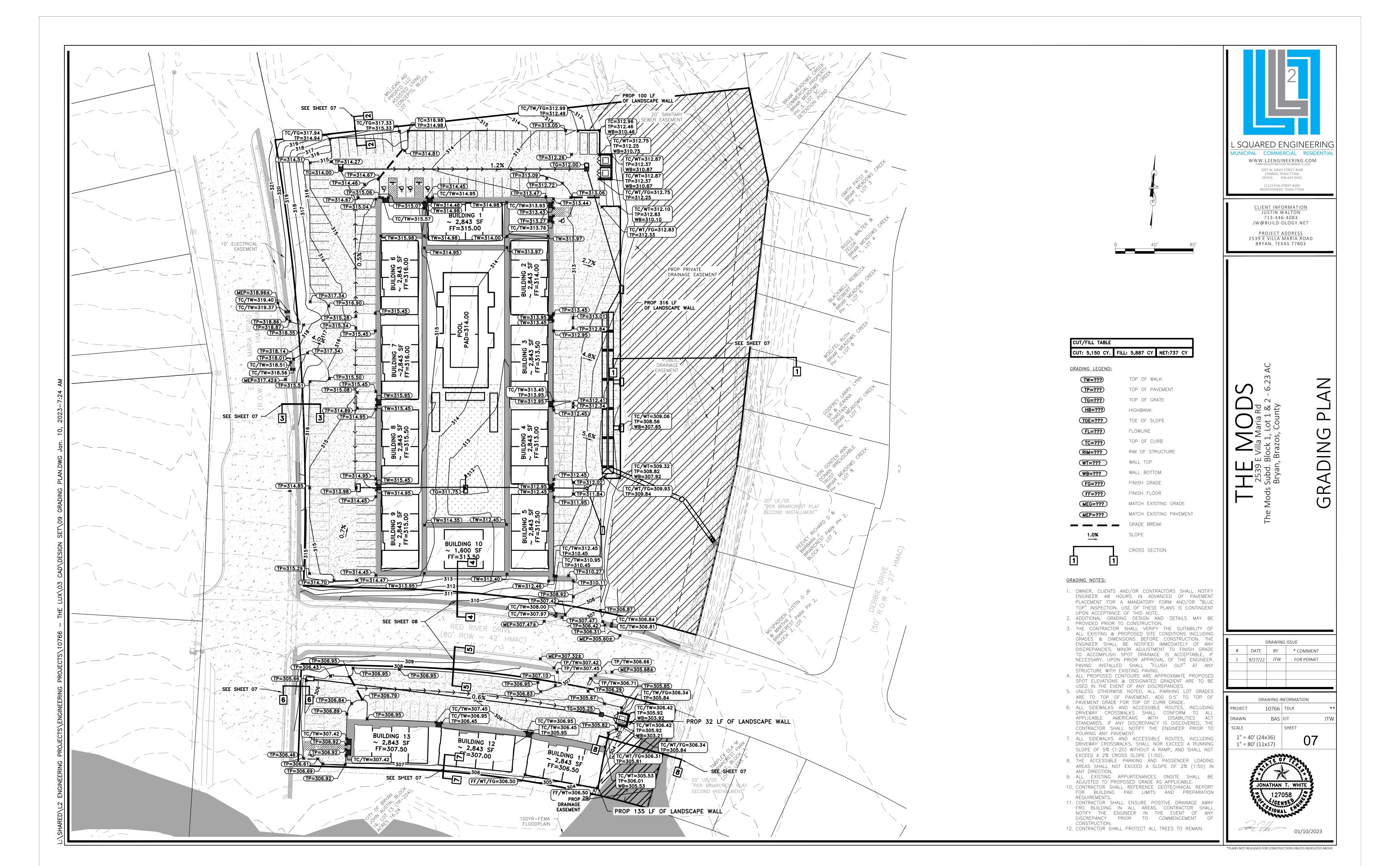
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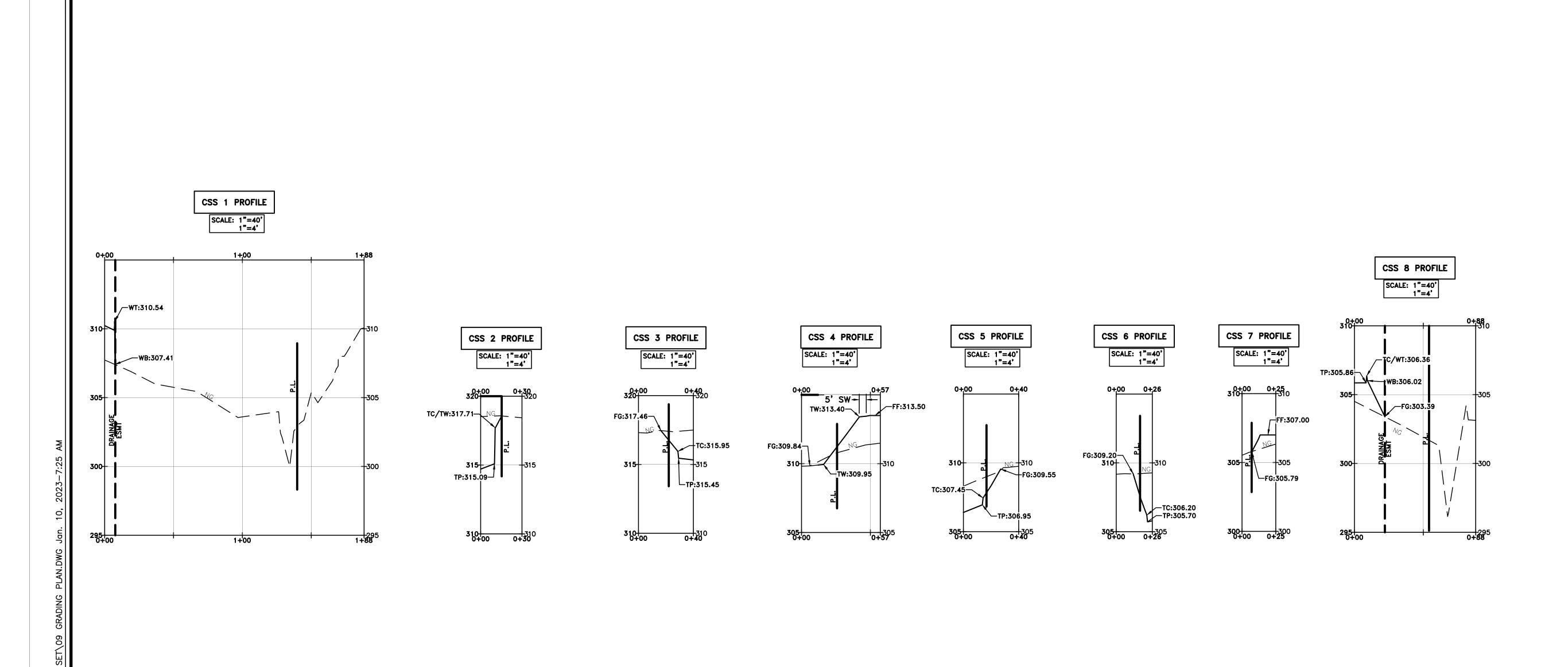
<u>SIO</u>

DRAWING ISSUE										
#	DATE	BY	* COMMENT							
1	9/27/22	JTW	FOR PERMIT							

	DRAWING II	NFORMATION					
PROJECT	10766	TDLR	**				
DRAWN	BAS	EIT	JTW				
SCALE		SHEET					
1" = 40' 1" = 80'	` '	06					
	****						







# THE MOD

2539 E Villa Maria Rd 2539 E Villa Maria Rd The Mods Subd. Block 1, Lot 1 & Bryan, Brazos, County

L SQUARED ENGINEERING

MUNICIPAL COMMERCIAL RESIDENTIAL

WWW.L2ENGINEERING.COM
FIRM REGISTRATION NUMBER 11235

3307 W. DAVIS STREET #100 CONROE, TEXAS 77304 OFFICE: 936-647-0420

21123 EVA STREET #200 MONTGOMERY, TEXAS 77356

CLIENT INFORMATION
JUSTIN WALTON

713-446-4083 JW@BUILD-OLOGY.NET

PROJECT ADDRESS 2539 E VILLA MARIA ROAD BRYAN, TEXAS 77803

# DRAWING ISSUE # DATE BY \* COMMENT 1 9/27/22 JTW FOR PERMIT

	· ·									
DRAWING INFORMATION										
ROJECT	10766	TDLR	**							
RAWN	BAS	EIT	JTW							
SCALE		SHEET								
1" = 40'	(24x36)	$\cap$	<b>)</b>							

1" = 40' (24x36) 1" = 80' (11x17)

JONATHAN T. WHITE

127058

CENS

ONAL

01/10/2023

\*PLANS NOT RELEASED FOR CONSTRUCTION UNLESS INDICATED ABOVE

#### GRADING NOTES:

**GRADING LEGEND:** 

TW=???

TP=???

TG=???

(HB=???)

TOE=???

FL=???

TC=???

RIM=???

WALL=???

FG=???

(FF=???)

MEG=???

TOP OF WALK

TOP OF GRATE

TOE OF SLOPE

TOP OF CURB

RIM OF STRUCTURE

MATCH EXISTING GRADE

MATCH EXISTING PAVEMENT

HIGHBANK

FLOWLINE

WALL TOP

FINISH GRADE

FINISH FLOOR

GRADE BREAK

CROSS SECTION

SLOPE

TOP OF PAVEMENT

- OWNER, CLIENTS AND/OR CONTRACTORS SHALL NOTIFY ENGINEER 48 HOURS IN ADVANCED OF PAVEMENT PLACEMENT FOR A MANDATORY FORM AND/OR "BLUE TOP" INSPECTION. USE OF THESE PLANS IS CONTINGENT UPON ACCEPTANCE OF THIS NOTE.
   ADDITIONAL GRADING DESIGN AND DETAILS MAY BE
- UPON ACCEPTANCE OF THIS NOTE.

  2. ADDITIONAL GRADING DESIGN AND DETAILS MAY BE PROVIDED PRIOR TO CONSTRUCTION.

  3. THE CONTRACTOR SHALL VERIFY THE SUITABILITY OF ALL EXISTING & PROPOSED SITE CONDITIONS INCLUDING GRADES & DIMENSIONS BEFORE CONSTRUCTION. THE
- GRADES & DIMENSIONS BEFORE CONSTRUCTION. THE ENGINEER SHALL BE NOTIFIED IMMEDIATELY OF ANY DISCREPANCIES. MINOR ADJUSTMENT TO FINISH GRADE TO ACCOMPLISH SPOT DRAINAGE IS ACCEPTABLE, IF NECESSARY, UPON PRIOR APPROVAL OF THE ENGINEER. PAVING INSTALLED SHALL "FLUSH OUT" AT ANY STRUCTURE WITH EXISTING PAVING.

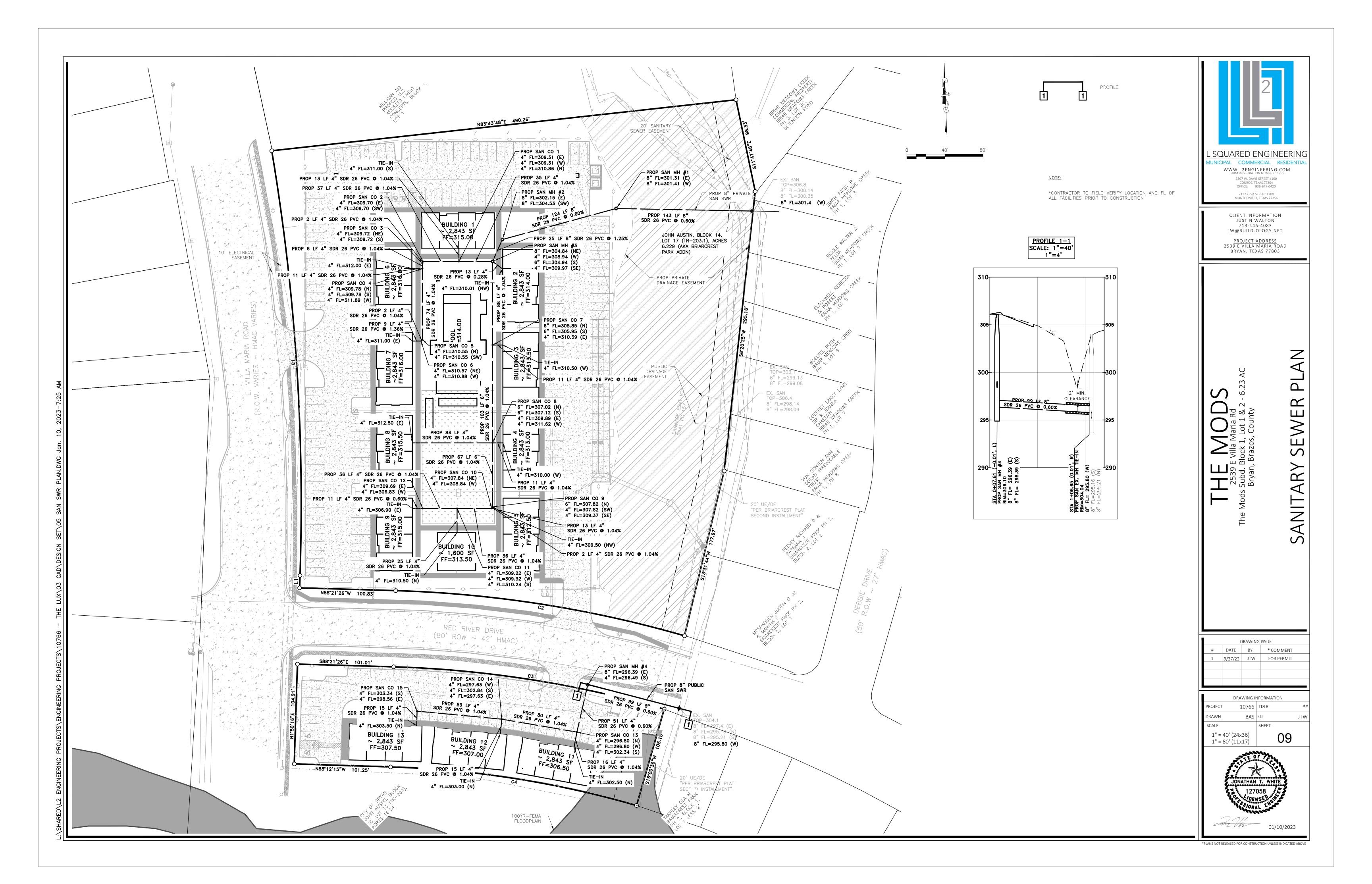
  4. ALL PROPOSED CONTOURS ARE APPROXIMATE PROPOSED SPOT ELEVATIONS & DESIGNATED GRADIENT ARE TO BE
- 4. ALL PROPOSED CONTOURS ARE APPROXIMATE PROPOSED SPOT ELEVATIONS & DESIGNATED GRADIENT ARE TO BE USED IN THE EVENT OF ANY DISCREPANCIES.

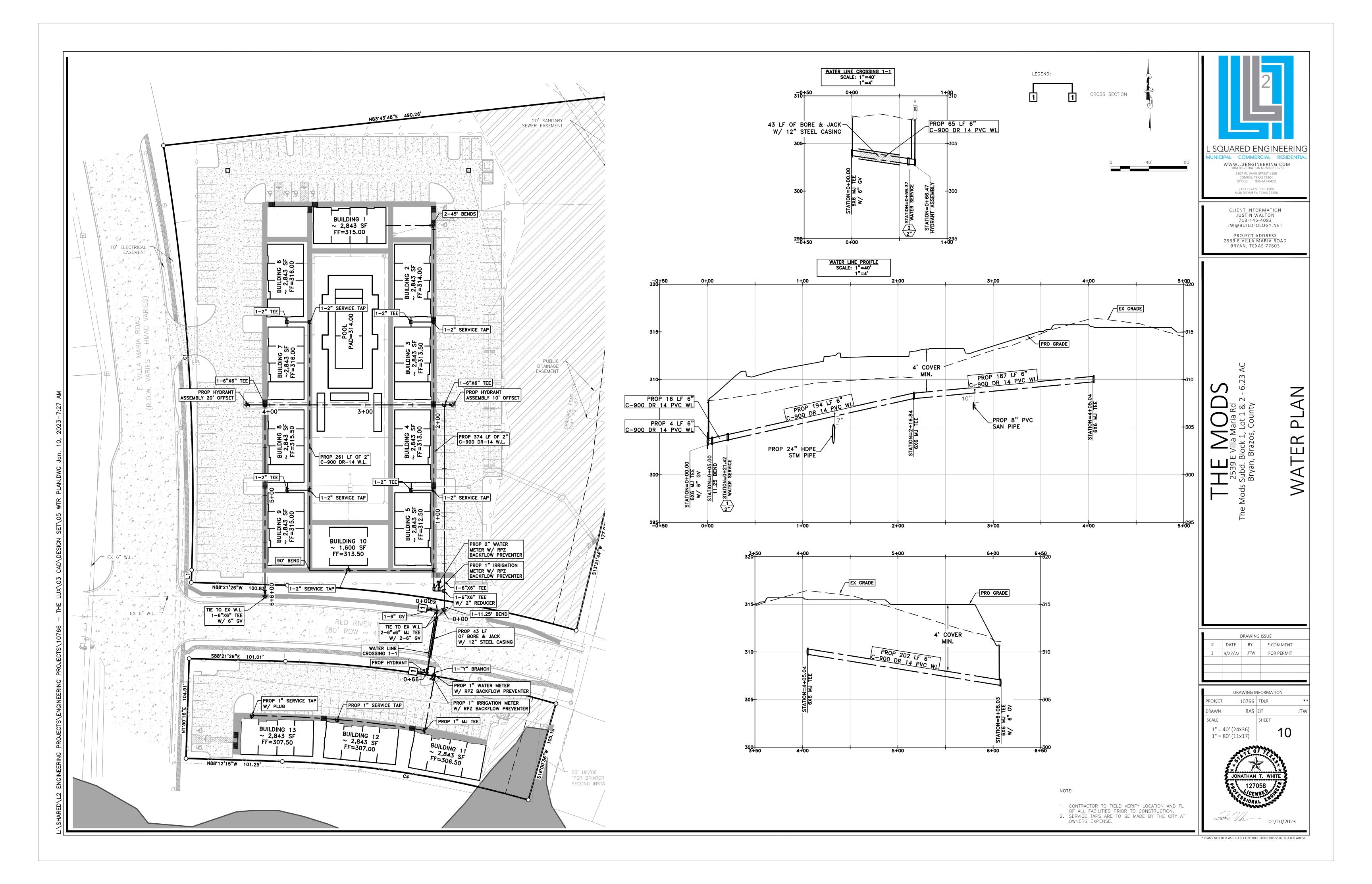
  5. UNLESS OTHERWISE NOTED, ALL PARKING LOT GRADES ARE TO TOP OF PAVEMENT. ADD 0.5' TO TOP OF PAVEMENT GRADE FOR TOP OF CURB GRADE.
- ALL SIDEWALKS AND ACCESSIBLE ROUTES, INCLUDING DRIVEWAY CROSSWALKS SHALL CONFORM TO ALL APPLICABLE AMERICANS WITH DISABILITIES ACT STANDARDS. IF ANY DISCREPANCY IS DISCOVERED, THE CONTRACTOR SHALL NOTIFY THE ENGINEER PRIOR TO POURING ANY PAVEMENT.
   ALL SIDEWALKS AND ACCESSIBLE ROUTES, INCLUDING DRIVEWAY CROSSWALKS, SHALL NOR EXCEED A RUNNING
- DRIVEWAY CROSSWALKS, SHALL NOR EXCEED A RUNNING SLOPE OF 5% (1:20) WITHOUT A RAMP, AND SHALL NOT EXCEED A 2% CROSS SLOPE (1:50).

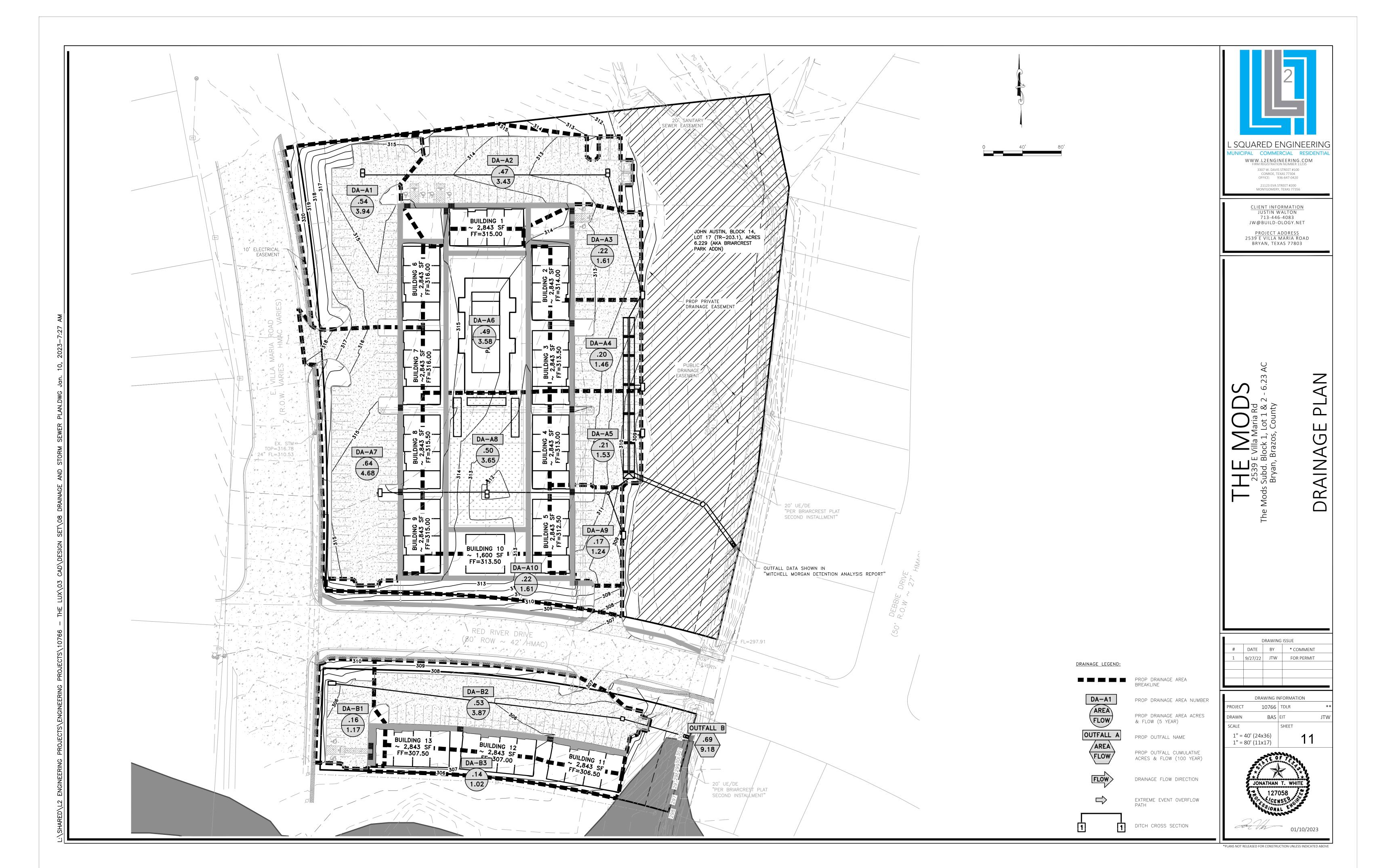
  8. THE ACCESSIBLE PARKING AND PASSENGER LOADING
- AREAS SHALL NOT EXCEED A SLOPE OF 2% (1:50) IN ANY DIRECTION.

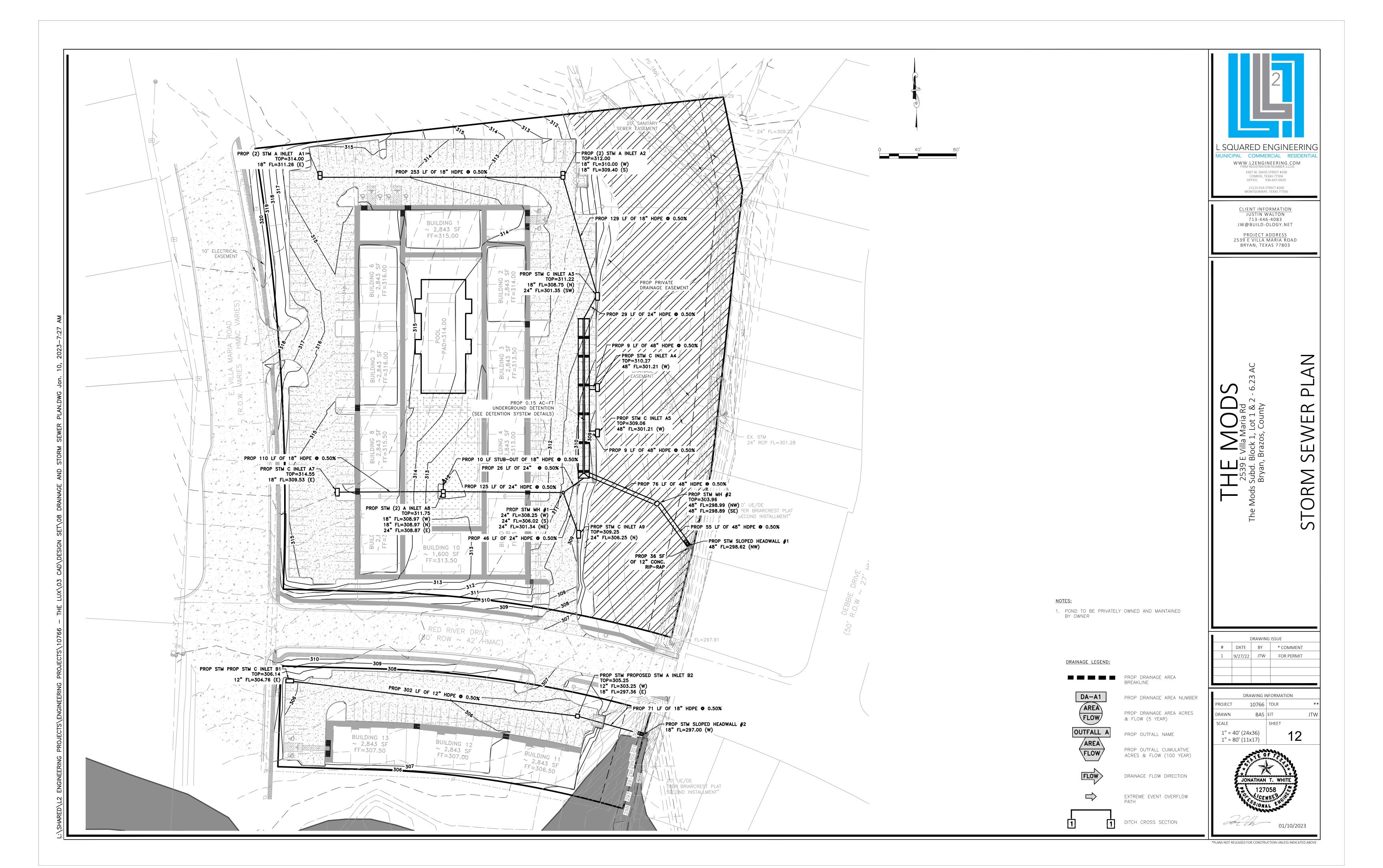
  9. ALL EXISTING APPURTENANCES ONSITE SHALL BE ADJUSTED TO PROPOSED GRADE AS APPLICABLE.
- 10. CONTRACTOR SHALL REFERENCE GEOTECHNICAL REPORT FOR BUILDING PAD LIMITS AND PREPARATION REQUIREMENTS.
  11. CONTRACTOR SHALL ENSURE POSITIVE DRAINAGE AWAY FRO BUILDING IN ALL AREAS. CONTRACTOR SHALL NOTIFY THE ENGINEER IN THE EVENT OF ANY DISCREPANCY PRIOR TO COMMENCEMENT OF
- DISCREPANCY PRIOR TO COMMENCEMENT CONSTRUCTION.

  12. CONTRACTOR SHALL PROTECT ALL TREES TO REMAIN.









5 Year																												_	
<u>Inlet Info</u>												_												HGL					
		Drainage	Total Drainage	Runoff	Co.	DA		Total Time of Conc.		Drainage Area Flow	Total Flow	Number of	Diameter			Perimeter		Length	Roughness	Q capacity	$V_{fullflow}$	Upstream	Downstream			Elevation of Hyd. Grad.	Elevation of Hyd. Grad.	5-YR CAPACIT	SOFFIT
Inlet/MH From In	let/MH To	Area	Area	"C"		C*A Tota	l C*A	(Min)	Intensity (I)	(cfs)	(cfs)	Barrels	(ft)	Slope	Area (A)	(P)	R=(A/P)	(ft)	(n)	(CFS)	(FPS)	FL	FL	Head (ft)		•	Downstream(ft)	СНЕСК	
Inlet A1	Inlet A	.2 0.5	4 0.	54 (	).95	0.51	0.51	10.00	7.69	3.93	3.9	3 1	1.5	0.005	1.77	4.7	0.375	25	3 0.011	8.78	4.97	311.26	310.00	0.25	0.10	311.75	311.50	YES	YES
Inlet A2	Inlet A	.3 0.4	7 1.	01 (	).95	0.44	0.95	10.00	7.69	3.42	2. 7.3	1 1	1.5	0.005	1.77	4.7	0.375	12	9 0.011	8.78	4.97	309.40	308.75	0.45	0.35	310.70	310.25	YES	YES
Inlet A3	OU	T 0.2	2 1.	23 (	).95	0.21	1.16	10.00	7.69	1.60	8.9	1	2	0.005	3.14	6.28	3 0.5	2	9 0.011	18.90	6.02	301.78	301.64	0.03	0.11	303.67	303.64	YES	YES
Inlet A7	Inlet A	.8 0.6	4 0.	64 (	).95	0.60	0.60	10.00	7.69	4.65	4.6	5 1	1.5	0.005	1.77	4.7	0.375	11	0 0.011	8.78	4.97	309.05	308.49	0.15	0.14	310.17	310.02	YES	YES
Stub Out	Inlet A	.8 0.4	9 1.	13 (	).95	0.46	1.07	10.00	7.69	3.56	8.2	1 1	1.5	0.005	1.77	4.7	0.375	1	0 0.011	8.78	4.97	308.49	308.49	0.04	0.44	310.06	310.02	YES	YES
Inlet A8	МН	1 0.	5 1.	63 (	).95	0.47	1.54	10.00	7.69	3.63	11.8	5 1	2	0.005	3.14	6.28	8 0.5	12	5 0.011	18.90	6.02	308.39	307.77	0.25	0.20	310.02	309.77	YES	YES
Inlet A9	МН	1 0.1	7 1.	30 (	).95	0.16	1.23	10.00	7.69	1.24	9.4	5 1	2	0.005	3.14	6.28	8 0.5	4	6 0.011	18.90	6.02	309.25	307.02	0.06	0.12	309.08	309.02	YES	YES
MH 1	OU	T 0.1	7 1.	80 (	).95	0.16	1.70	10.00	7.69	1.24	13.09	1	2	0.005	3.14	6.28	8 0.5	2	6 0.011	18.90	6.02	306.55	306.42	0.06	0.24	308.48	308.42	YES	
																									Starting	TW Elevation:	308.42	2	

100 fear																												_		
Inlet Info																								HGL						
			Total							Drainage														⊢	ydraulic	Elevation of	Elevation of	CDITICAL	COFFIT	
		Drainage	Drainage	e Ru	ınoff Co.	DA		Time of		Area Flow	<b>Total Flow</b>	Number of	Diameter			Perimeter		Length	Roughness	Q capacity	$V_{fullflow}$	Upstream	Downstream	Change In G	radeline	Hyd. Grad.	Hyd. Grad.	CRITICAL ELEVATION	SOFFII O	vertopping?
Inlet/MH From Inle	t/MH To	Area	Area		"C"	C*A Tot	al C* <i>A</i>	Conc. (Min)	Intensity (I)	(cfs)	(cfs)	Barrels	(ft)	Slope	Area (A)	(P)	R=(A/P)	(ft)	(n)	(CFS)	(FPS)	FL	FL	Head (ft)	%	Upstream (ft)	Downstream(ft)	ELEVATION	CHECK	
Inlet A1	Inlet A2	2 0.5	4 0.	.54	0.95	0.51	0.51	1 10.00	11.64	5.94	5.94	1	1.5	0.005	1.77	4.71	0.375	253	0.011	8.78	4.97	311.26	310.00	0.58	0.23	311.86	311.28	314.00	NO	NO
Inlet A2	Inlet A	3 0.4	.7 1.	.01	0.95	0.44	0.95	5 10.00	11.64	5.17	11.11	1	1.5	0.005	1.77	4.71	0.375	129	0.011	8.78	4.97	309.40	308.75	1.03	0.80	311.28	310.25	312.00	YES	NO
Inlet A3	OU <sup>-</sup>	Г 0.2	2 1.	.23	0.95	0.21	1.16	5 10.00	11.64	2.42	13.53	1	2	0.005	3.14	6.28	0.5	29	0.011	18.90	6.02	301.78	301.64	0.07	0.25	305.71	305.64	311.22	YES	NO
Inlet A7	Inlet A	3 0.6	64 0.	.64	0.95	0.60	0.60	0 10.00	11.64	7.04	7.04	1	1.5	0.005	1.77	4.71	0.375	110	0.011	8.78	4.97	309.05	308.49	0.35	0.32	310.69	310.33	314.55	YES	NO
Stub Out	Inlet A	3 0.4	9 1.	.13	0.95	0.46	1.07	7 10.00	11.64	5.39	12.43	1	1.5	0.005	1.77	4.71	0.375	10	0.011	8.78	4.97	308.49	308.49	0.10	1.00	310.43	310.33	312.00	YES	NO
Inlet A8	MH:	1 0.	.5 1.	.63	0.95	0.47	1.54	4 10.00	11.64	5.50	17.93	1	2	0.005	3.14	6.28	0.5	125	0.011	18.90	6.02	308.39	307.77	0.56	0.45	310.33	309.77	311.75	YES	NO
Inlet A9	MH:	1 0.1	.7 1.	.30	0.95	0.16	1.23	3 10.00	11.64	1.87	14.30	1	2	0.005	3.14	6.28	0.5	46	0.011	18.90	6.02	309.25	307.02	0.13	0.28	309.15	309.02	309.25	YES	NO
MH 1	OU <sup>-</sup>	Γ 0.1	.7 1.	.80	0.95	0.16	1.70	0 10.00	11.64	1.87	19.80	1	2	0.005	3.14	6.28	0.5	26	0.011	18.90	6.02	306.55	306.42	0.14	0.55	308.56	308.42	312.00	YES	NO
																									Starting	TW Elevation:	308.42	<u> </u>		

#### SYSTEM B

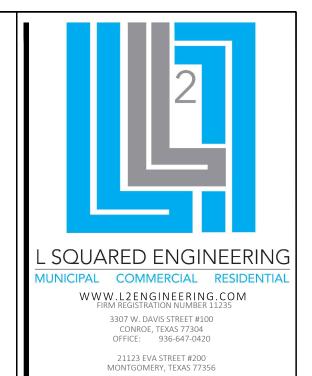
						<u> </u>	·																	
5 Year																								
<u>Inlet Info</u>																								
			Total								Drainage	,												
		Drainag <i>ę</i>	e Drainag	₃e Ru	unoff Co.	DA	Total	Time of			Area Flow	Total Flow	Number of	Diameter			Perimeter		Length	Roughness	$Q_{capacity}$	$V_{full\;flow}$	Upstream	Downstrean
Inlet/MH From Inle	t/MH To_	Area	Area		"C"	C*A	C*A	Conc. (Min)	Intensity (I)	Cf	(cfs)	(cfs)	Barrels	(ft)	Slope	Area (A)	(P)	R=(A/P)	(ft)	(n)	(CFS)	(FPS)	FL	FL
Inlet 12	Inlet B2	2 0.1	17 (	0.17	0.95	0.16	0.16	5 10.00	7.69	1.00	1.24	1.24	1	1	0.005	0.79	3.14	0.25	92	0.012	2.73	3.47	302.70	302.2
Inlet B2	Inlet B3	3 0.	.16 0	0.33	0.95	0.15	0.31	1 10.00	7.69	1.00	1.17	2.41	. 1	1	0.005	0.79	3.14	0.25	89	0.012	2.73	3.47	302.24	301.
Inlet B3	Inlet B4	<del>1</del> 0.	.16 0	0.49	0.95	0.15	0.47	7 10.00	7.69	1.00	1.17	3.58	. 1	1.5	0.005	1.77	4.71	0.375	47	0.012	8.04	4.55	301.79	301.
Inlet B4	Inlet B4	4 0.0	04 (	0.53	0.95	0.04	0.50	10.00	7.69	1.00	0.29	3.87	1	1.5	0.005	1.77	4.71	0.375	60	0.012	8.04	4.55	301.56	301.
Inlet B4	OUT	T 0.0	07	0.60	0.95	0.07	0.57	7 10.00	7.69	1.00	0.51	4.38	, 1	1.5	0.005	1.77	4.71	0.375	32	0.012	8.04	4.55	301.26	301.

100	Year

<u>nlet Info</u>																							
			Total							Drainage													
		Drainage	Drainage	Runoff Co.	DA	Total	Time of			_	Total Flow	Number of	Diameter			Perimeter		Length	Roughness	Q capacity	$V_{full\ flow}$	Upstream	Downstream
nlet/MH From Inle	t/MH To	Area	Area	"C"	C*A	C*A	Conc. (Min)	Intensity (I)	Cf	(cfs)	(cfs)	Barrels	(ft)	Slope	Area (A)	(P)	R=(A/P)	(ft)	(n)	(CFS)	(FPS)	FL	FL
Inlet 12	Inlet B2	2 0.17	7 0.1	7 0.95	0.16	0.16	10.00	11.64	1.25	2.35	2.35	1	1	0.005	0.79	3.14	0.25	92	0.012	2.73	3.47	302.70	302.2
Inlet B2	Inlet B3	0.16	5 0.33	3 0.95	0.15	0.31	10.00	11.64	1.25	2.21	4.56	1	1	0.005	0.79	3.14	0.25	89	0.012	2.73	3.47	302.24	301.7
Inlet B3	Inlet B4	0.16	5 0.49	9 0.95	0.15	0.47	10.00	11.64	1.25	2.21	6.77	1	1.5	0.005	1.77	4.71	0.375	47	0.012	8.04	4.55	301.79	301.5
Inlet B4	Inlet B5	0.04	4 0.53	3 0.95	0.04	0.50	10.00	11.64	1.25	0.55	7.33	1	1.5	0.005	1.77	4.71	0.375	60	0.012	8.04	4.55	301.56	301.2
Inlet B5	OUT	0.07	7 0.60	0 0.95	0.07	0.57	7 10.00	11.64	1.25	0.97	8.29	1	1.5	0.005	1.77	4.71	0.375	32	0.012	8.04	4.55	301.26	301.1

#### **DETENTION NOTES:**

0.15 AC-FT OF DETENTION PROVIDED VIA UNDERGROUND DETENTION PER "THE MODS DETENTION ANALYSIS, OCT 2022" BY MITCHELL & MORGAN.



CLIENT INFORMATION
JUSTIN WALTON
713-446-4083
JW@BUILD-OLOGY.NET

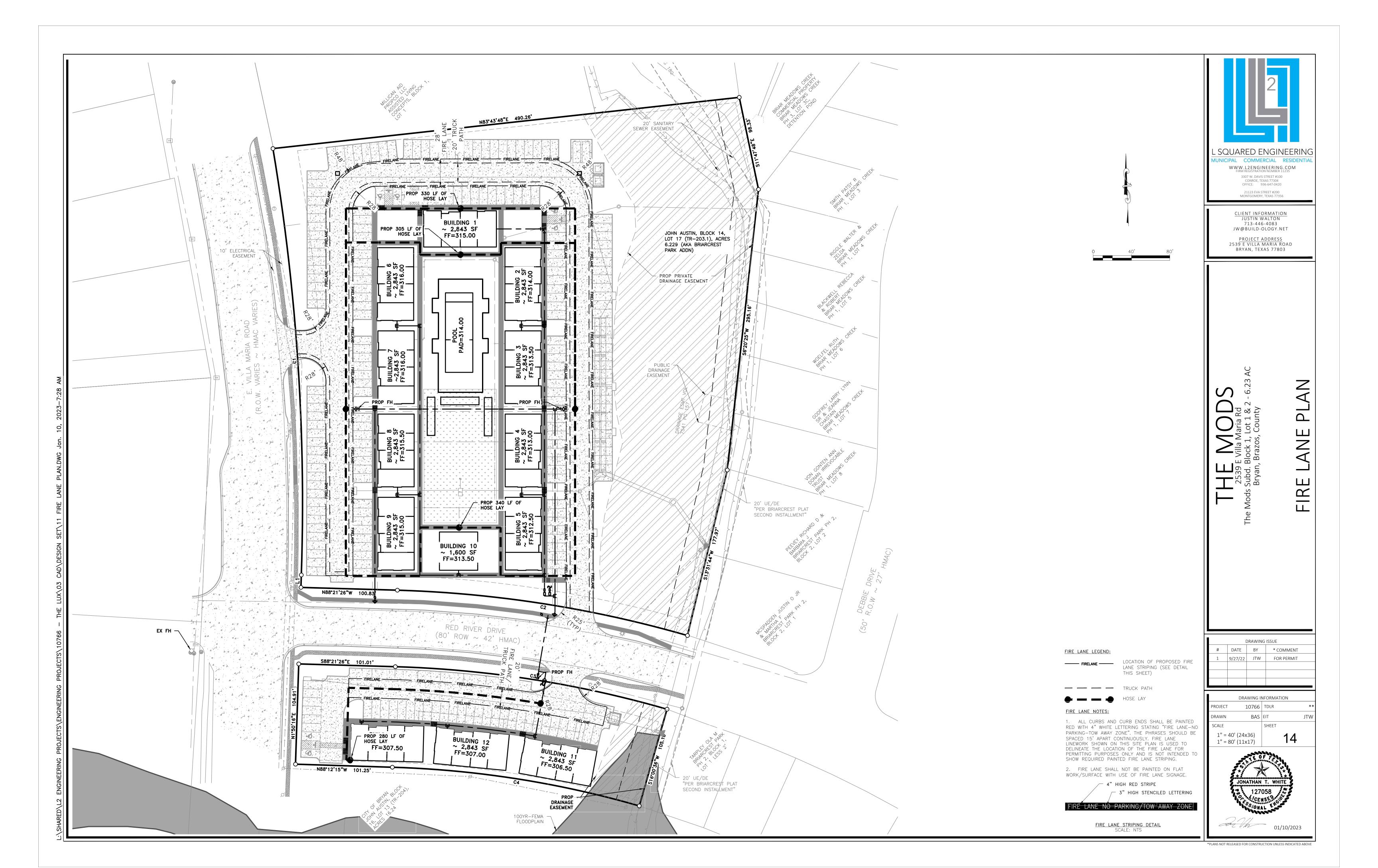
PROJECT ADDRESS 2539 E VILLA MARIA ROAD BRYAN, TEXAS 77803

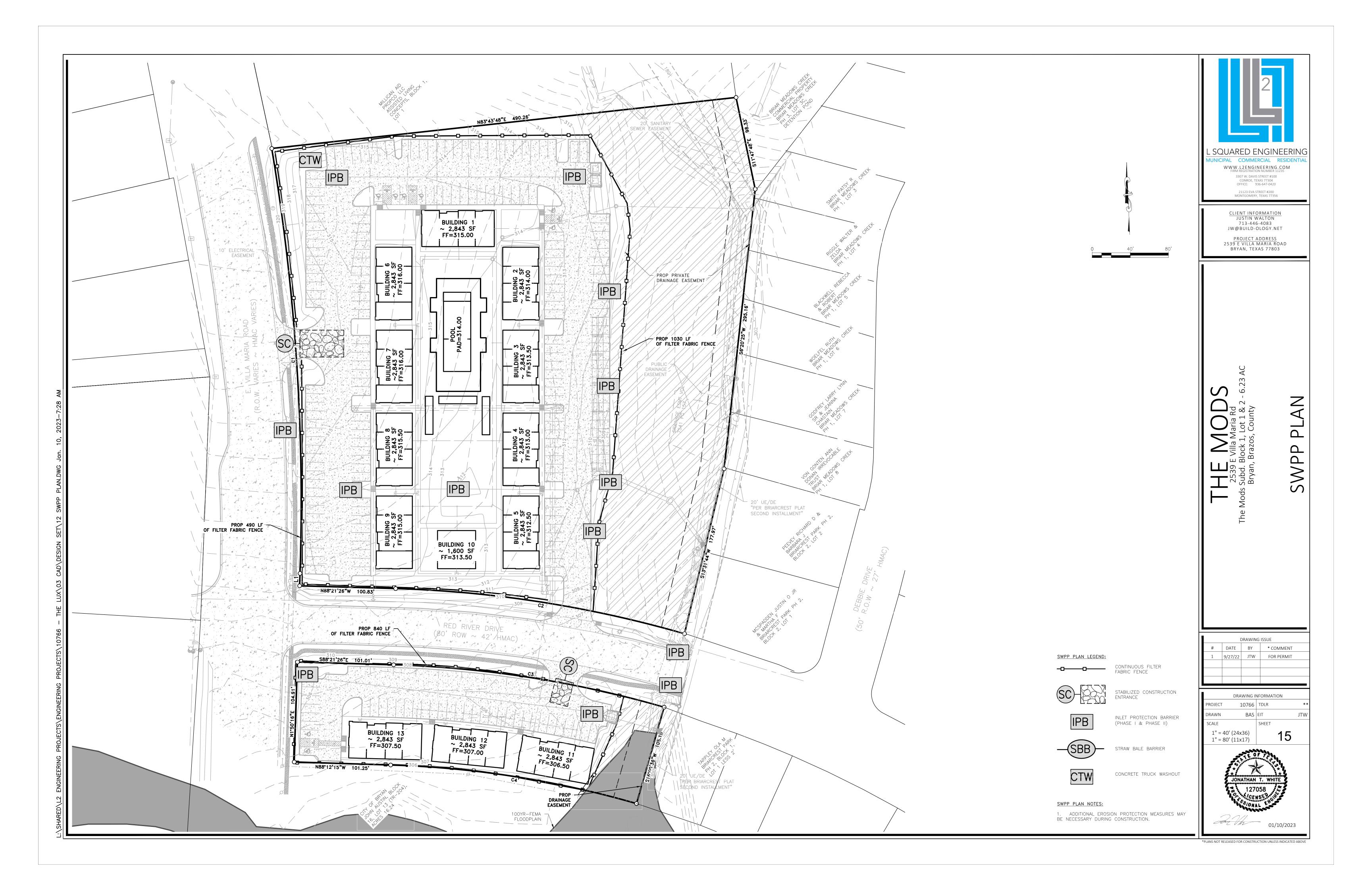
DRAWING ISSUE # DATE BY \* COMMENT 1 9/27/22 JTW FOR PERMIT

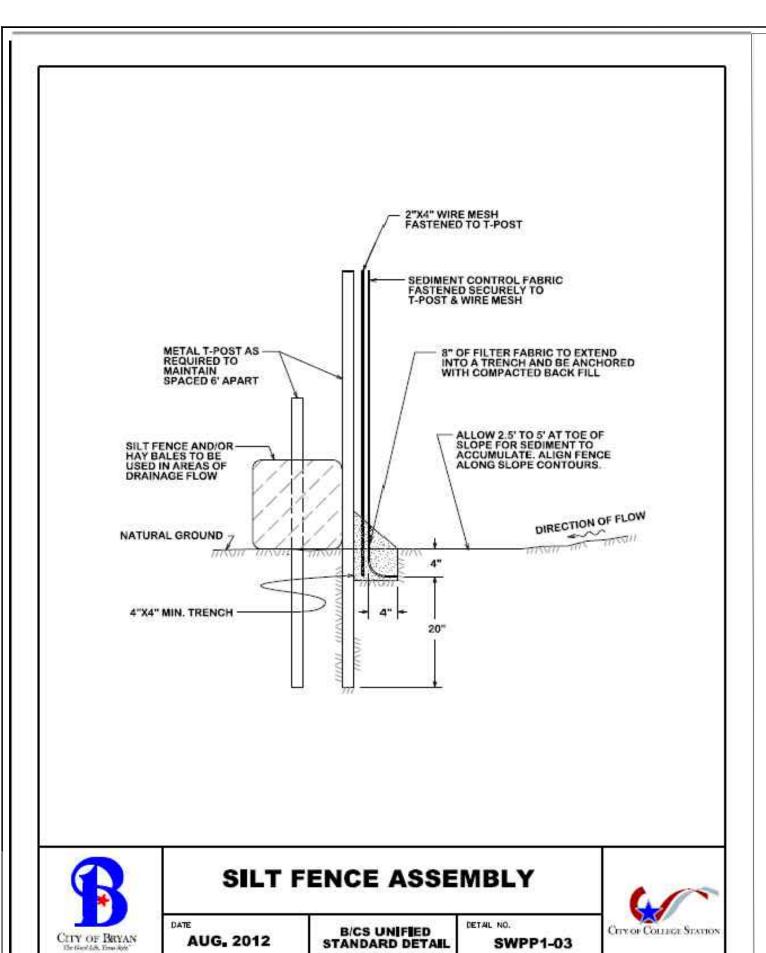
1		DRAWING II	NFORMATION	I
	PROJECT	10766	TDLR	**
	DRAWN	BAS	EIT	JTW
	SCALE		SHEET	
			1	3



\*PLANS NOT RELEASED FOR CONSTRUCTION UNLESS INDICATED ABOVE

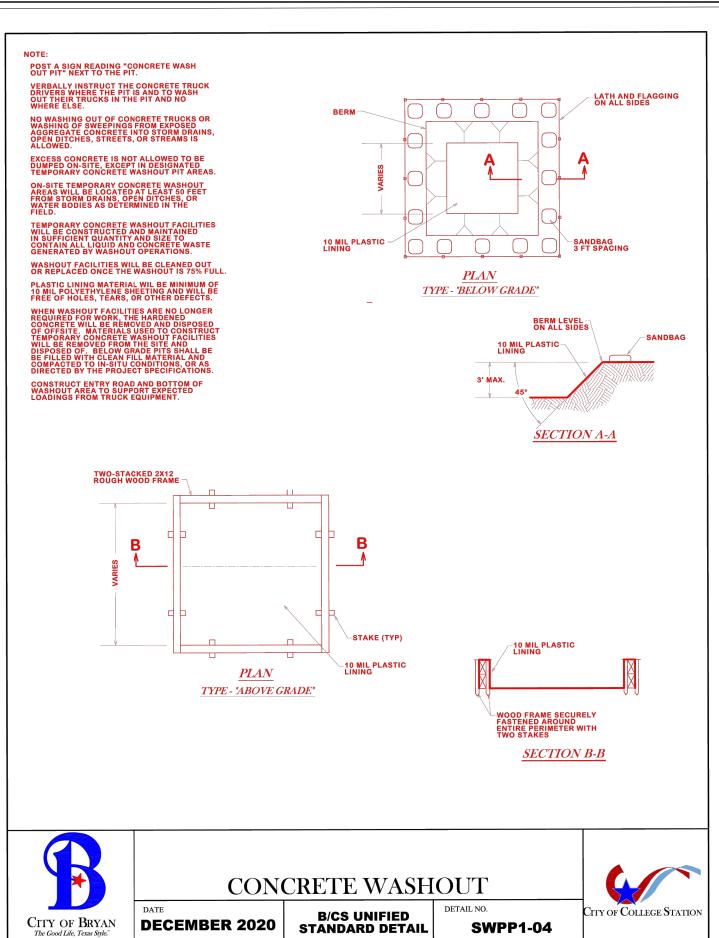




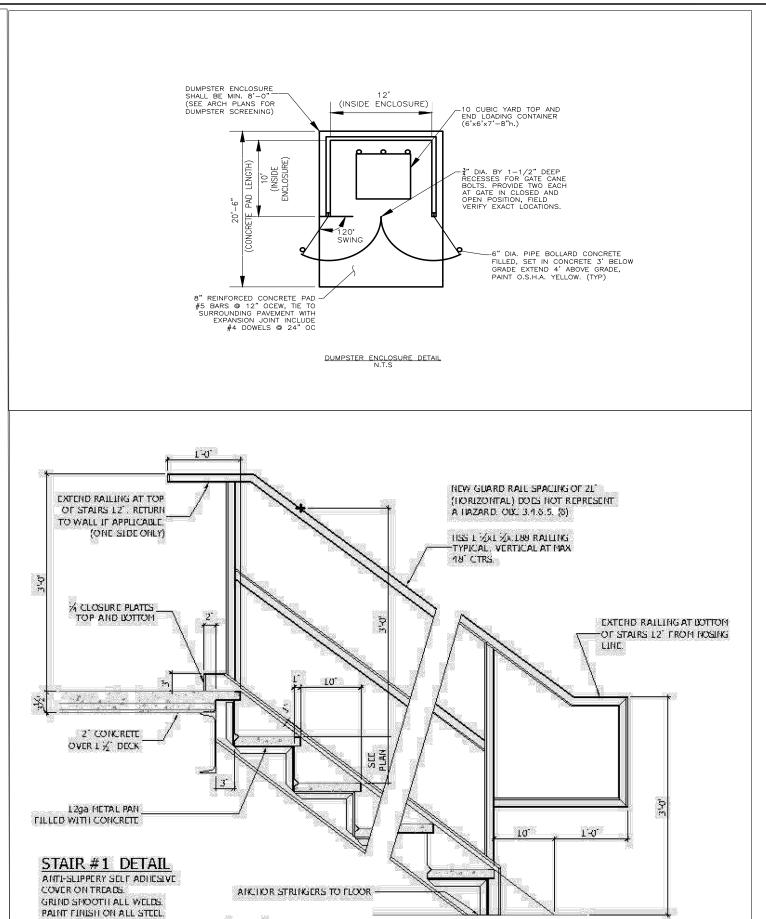


SWPP1-03

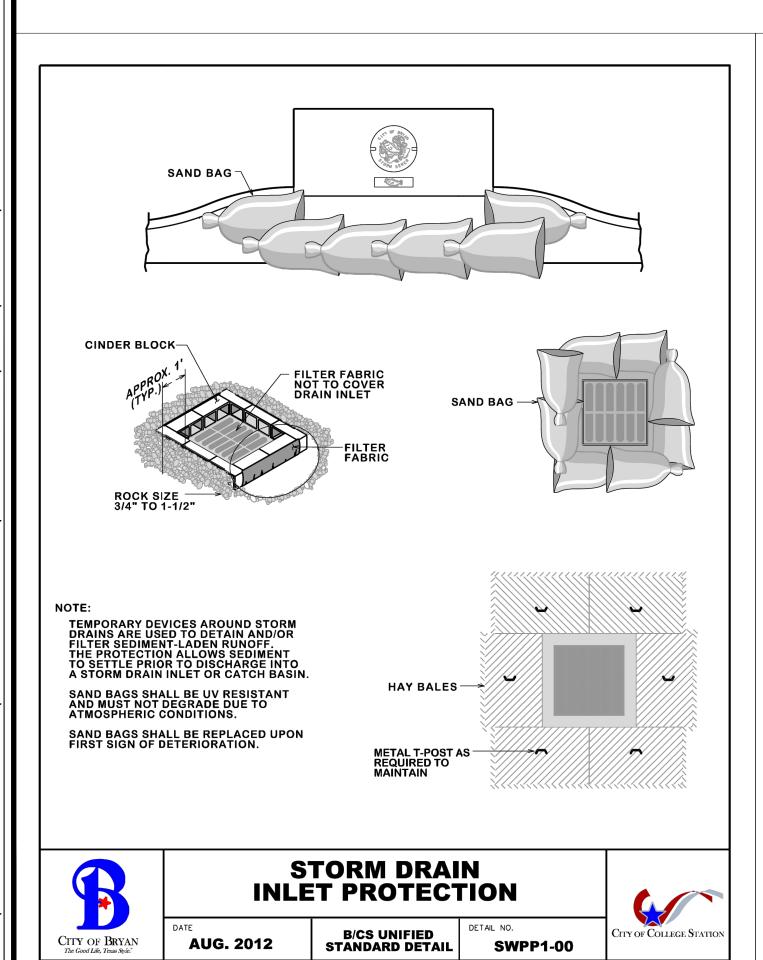
CITY OF BRYAN
The Good Life, Texas Style."

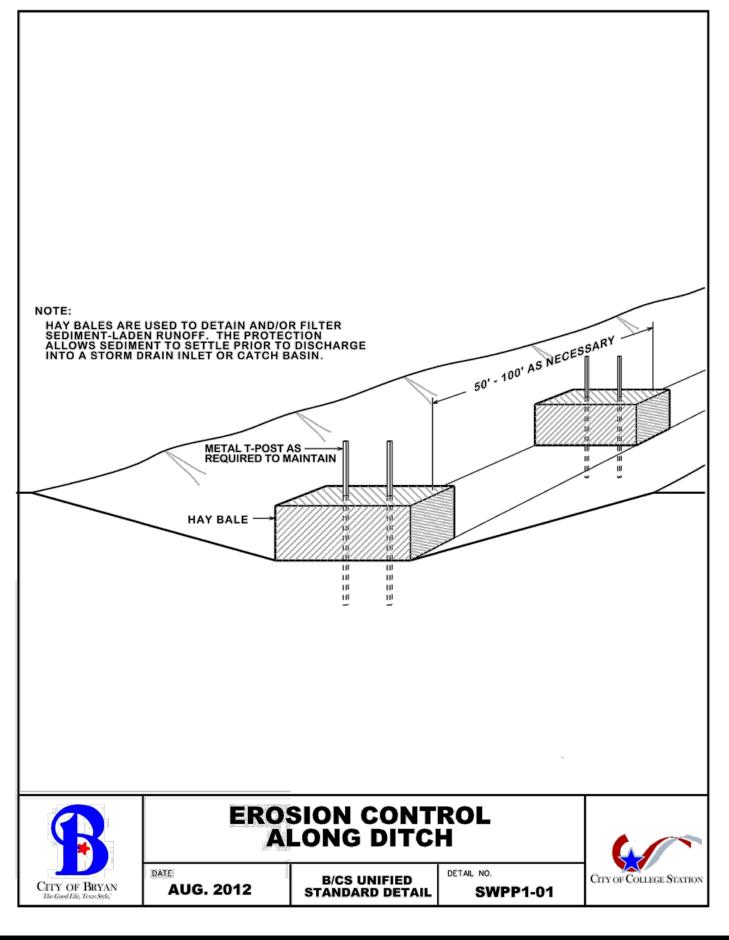


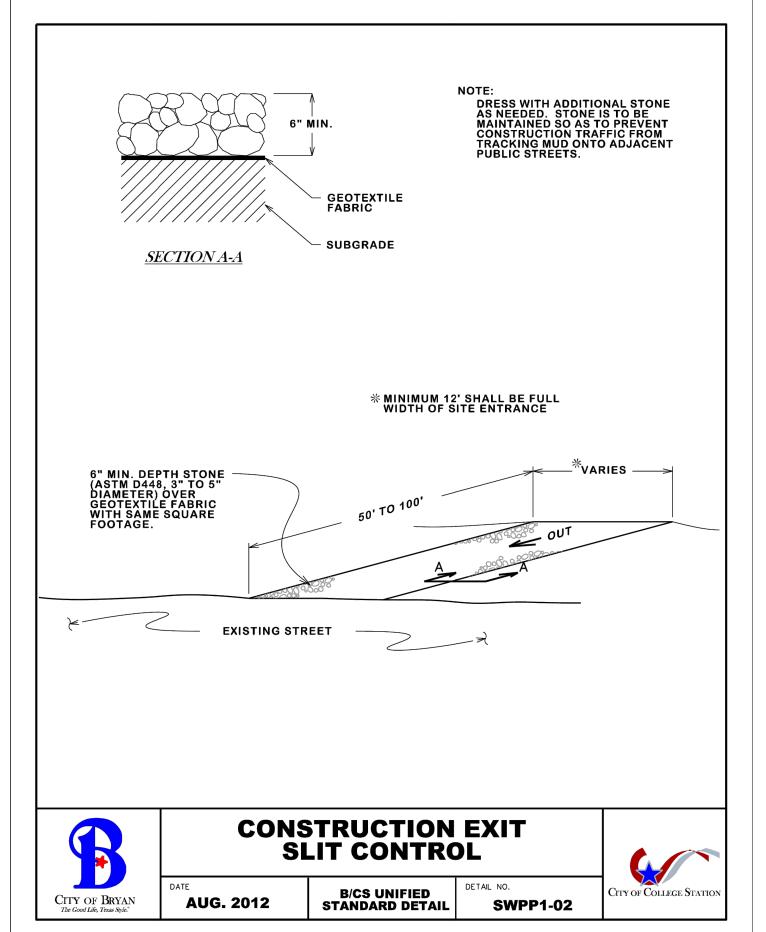
**SWPP1-04** 



PROVIDE 5'8%' KICK PLATE AT LANGINGS (TYPICAL)









CLIENT INFORMATION
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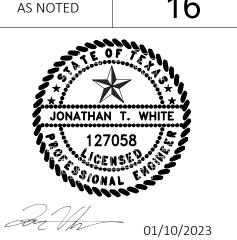
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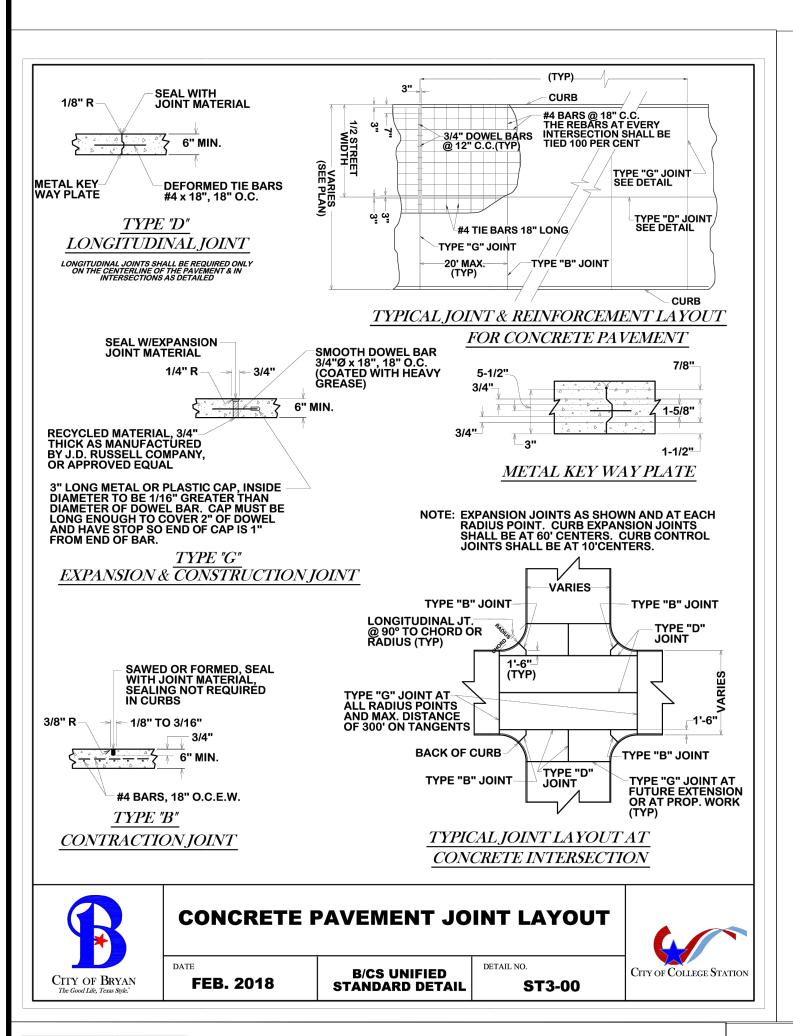
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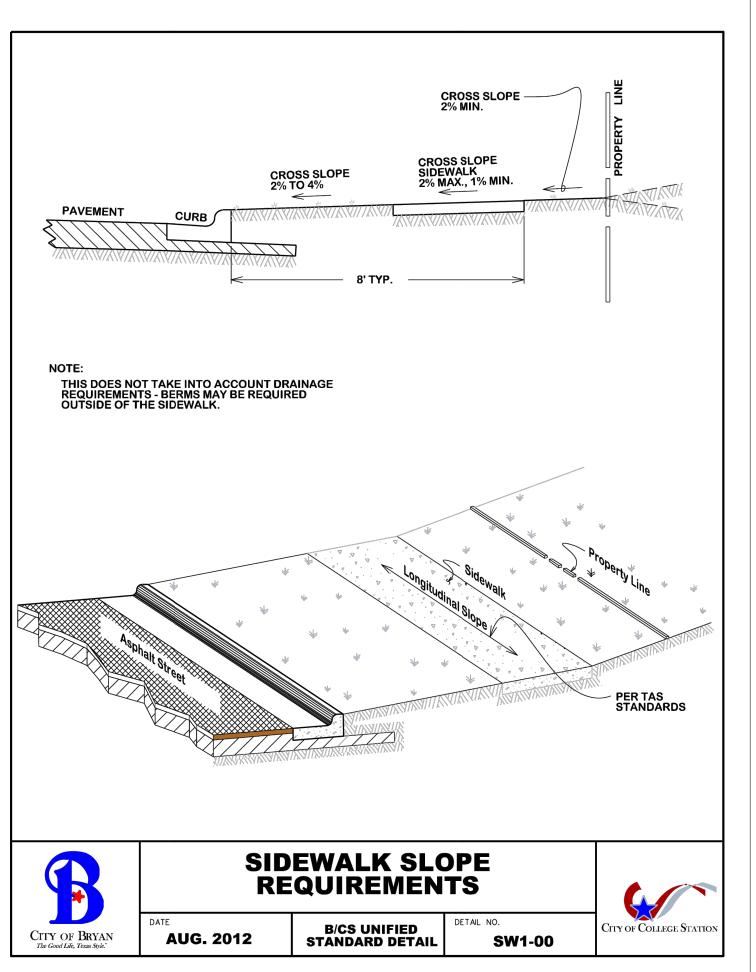
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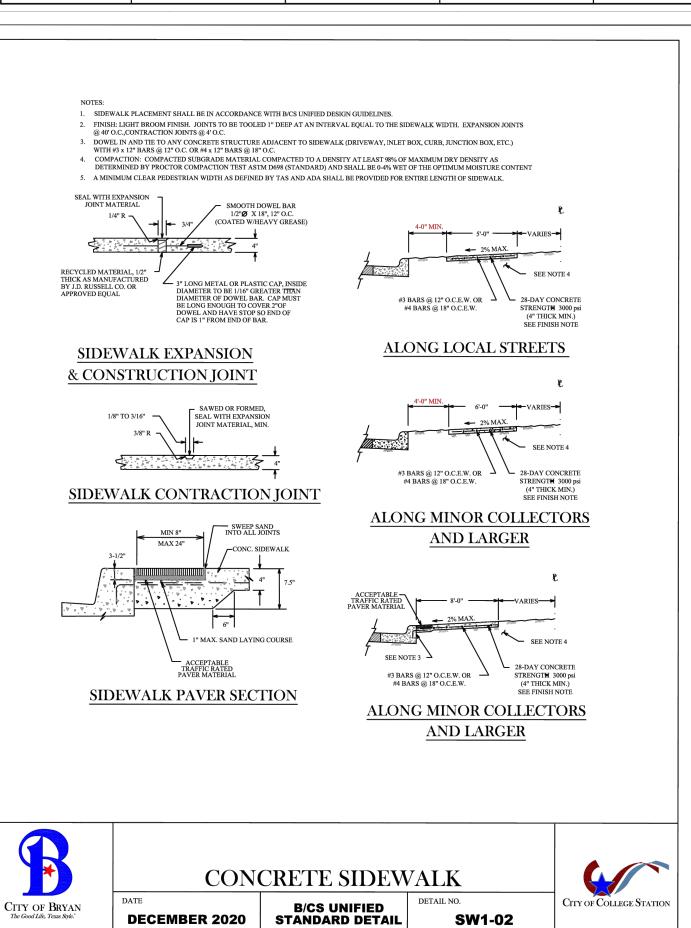
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		[	DRAWING	ISSUE
	#	DATE	BY	* COMMENT
	1	9/27/22	JTW	FOR PERMIT

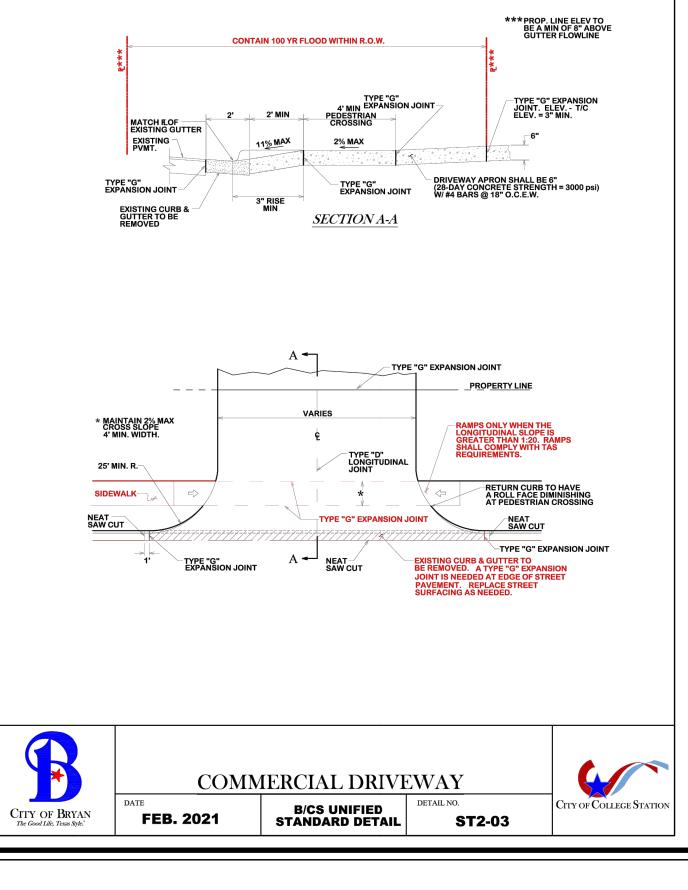
	DRAWING II	NFORMATION	
PROJECT	10766	TDLR	**
DRAWN	BAS	EIT	JTW
SCALE		SHEET	
AS N	OTED	1	6
		5050	

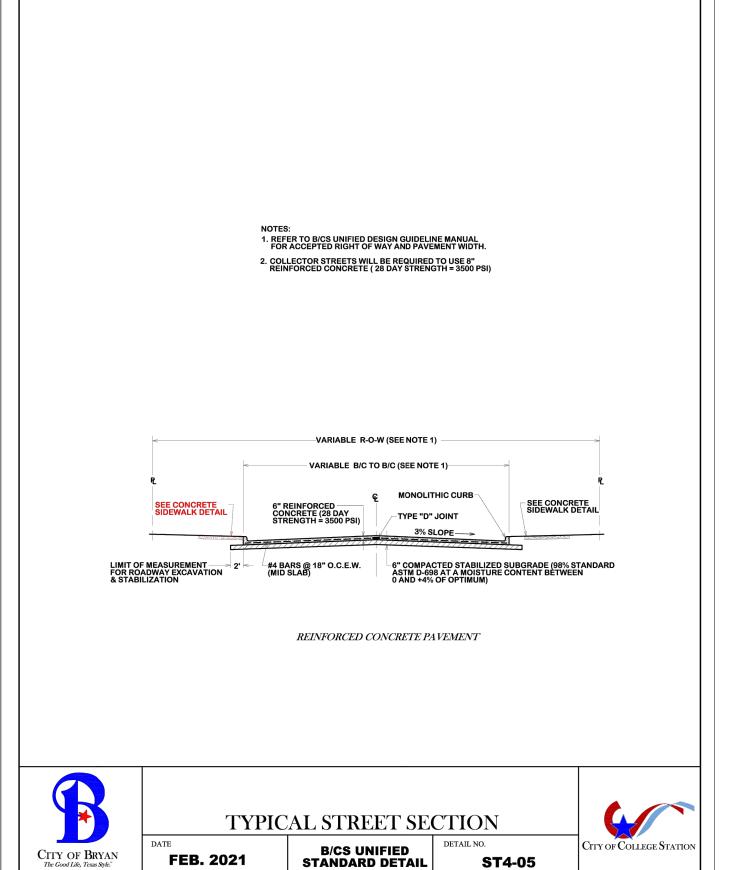














2539 E Villa Maria The Mods Subd. Block 1, Lot 1 Bryan, Brazos, Cour  $\overline{\triangleleft}$ 

<u>(</u>

L SQUARED ENGINEERING

IUNICIPAL COMMERCIAL RESIDENTIA

WWW.L2ENGINEERING.COM FIRM REGISTRATION NUMBER 11235 3307 W. DAVIS STREET #100 CONROE, TEXAS 77304 OFFICE: 936-647-0420

> 21123 EVA STREET #200 MONTGOMERY, TEXAS 77356

CLIENT INFORMATION
JUSTIN WALTON

713-446-4083

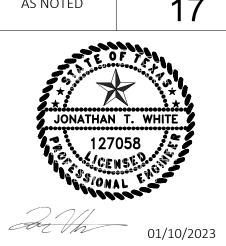
JW@BUILD-OLOGY.NET

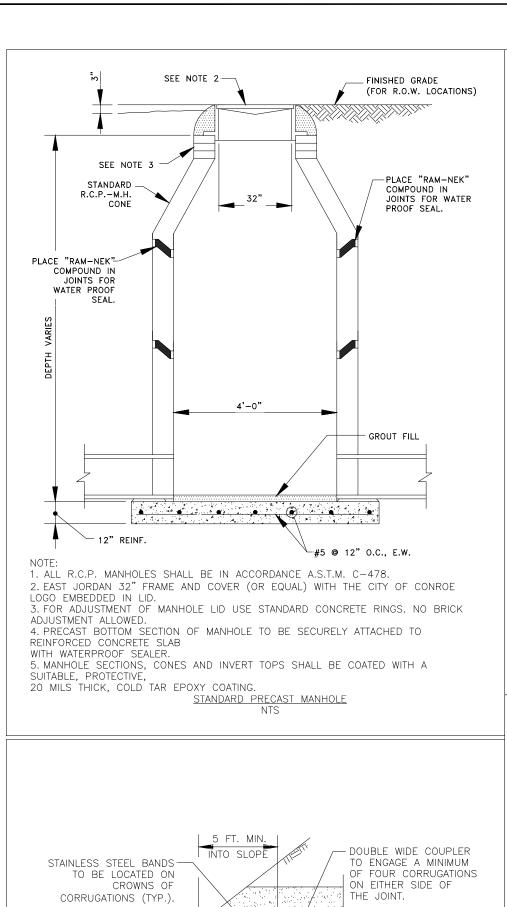
PROJECT ADDRESS 2539 E VILLA MARIA ROAD

BRYAN, TEXAS 77803

	D	RAWING	ISSUE
#	DATE	BY	* COMMENT
1	9/27/22	JTW	FOR PERMIT

	DRAWING II	NFORMATIC	Ν
PROJECT	10766	TDLR	**
DRAWN	BAS	EIT	JTW
SCALE		SHEET	
AS NO	OTED		17
	*****		





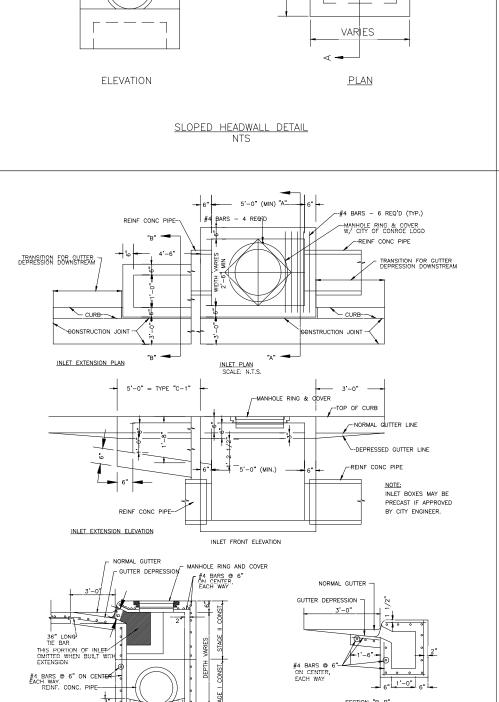
SEE NOTE 2.

BEDDING HDPE PIPE DETAIL NTS

CORRUGATIONS (TYP.).

HIGH DENSITY CORRUGATED —
POLYETHYLENE (HDPE)
SMOOTH LINED PIPE.

CEMENT STABILIZED — SAND ALL AROUND PIPE. SEE NOTE 2.



TYPE "C", "C-1" & "-2" INLETS

NTS

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SECTION A-A

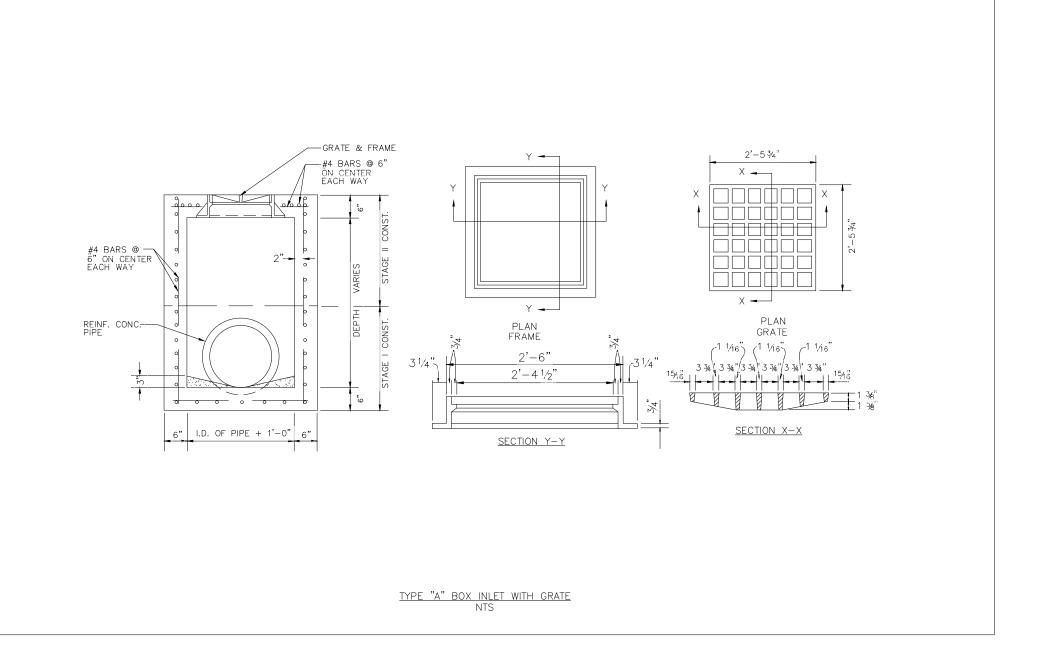
#3 BARS 12" O.C.E.W. 100% TIE

\_\_ \_\_ \_

SECTION "A-A"

\*INLETS MAY BE PRECAST

\_\_ \_\_ \_

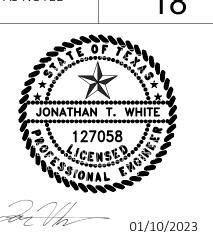




#	DATE	BY	* COMMENT	
1	9/27/22	JTW	FOR PERMIT	
	DRA	WING IN	NFORMATION	
PROJECT	-	10766	TDLR	*

DRAWING ISSUE

	DRAWING II	NFORMAT	ION	
PROJECT	10766	TDLR		*
DRAWN	BAS	EIT		JTV
SCALE		SHEET		
AS No	OTED		18	



AIL  $\propto$ STORM

L SQUARED ENGINEERING

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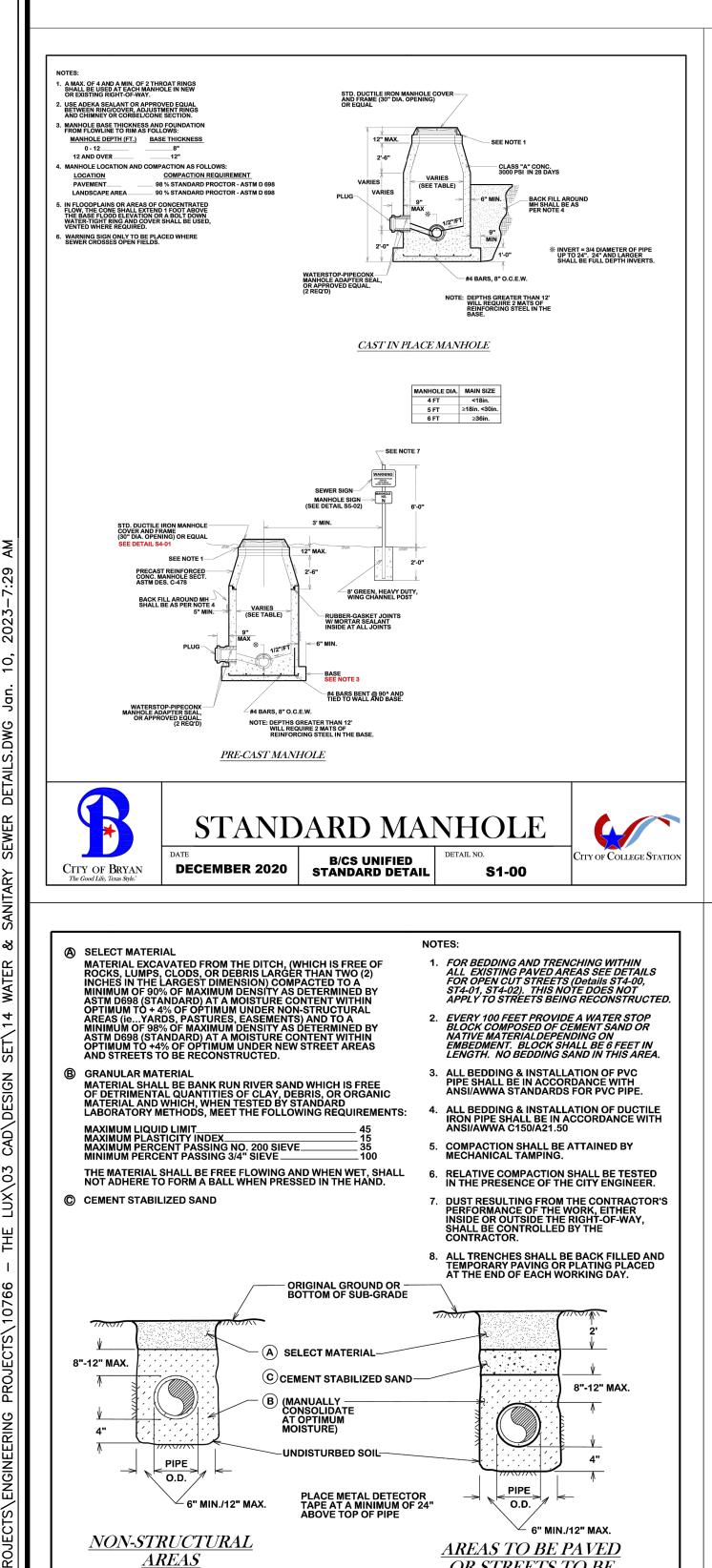
3307 W. DAVIS STREET #100 CONROE, TEXAS 77304 OFFICE: 936-647-0420

21123 EVA STREET #200 MONTGOMERY, TEXAS 77356

CLIENT INFORMATION
JUSTIN WALTON 713-446-4083

JW@BUILD-OLOGY.NET

PROJECT ADDRESS 2539 E VILLA MARIA ROAD BRYAN, TEXAS 77803



OR STREETS TO BE

RECONSTRUCTED

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TY OF COLLEGE STATIC

**BEDDING AND TRENCH FOR** 

DI PIPE & PVC PIPE WITHIN

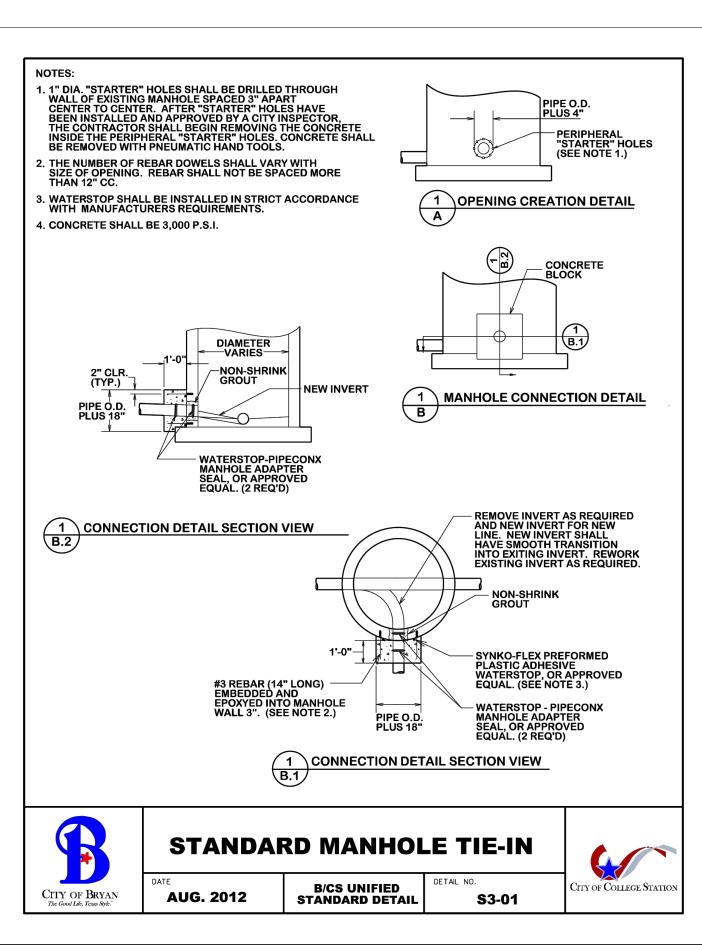
**NON-STRUCTURAL OR NEW** 

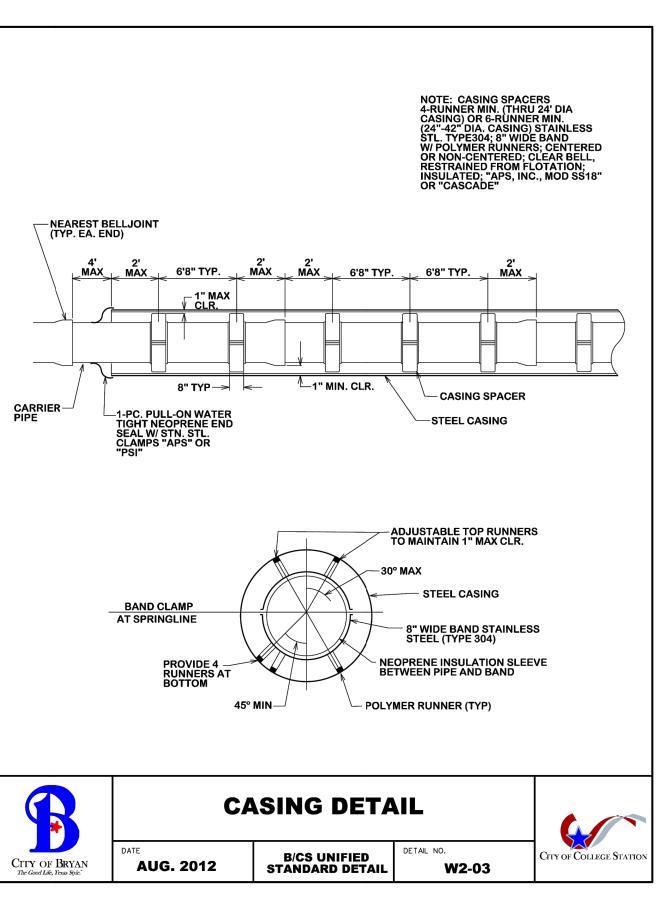
CITY OF BRYAN
The Good Life, Texas Style."

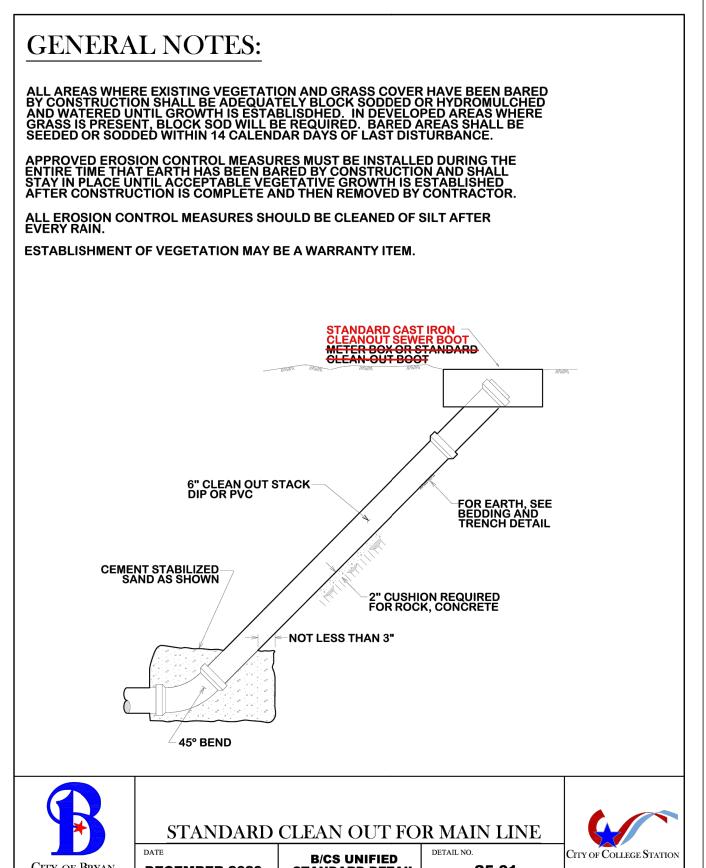
**AUG. 2012** 

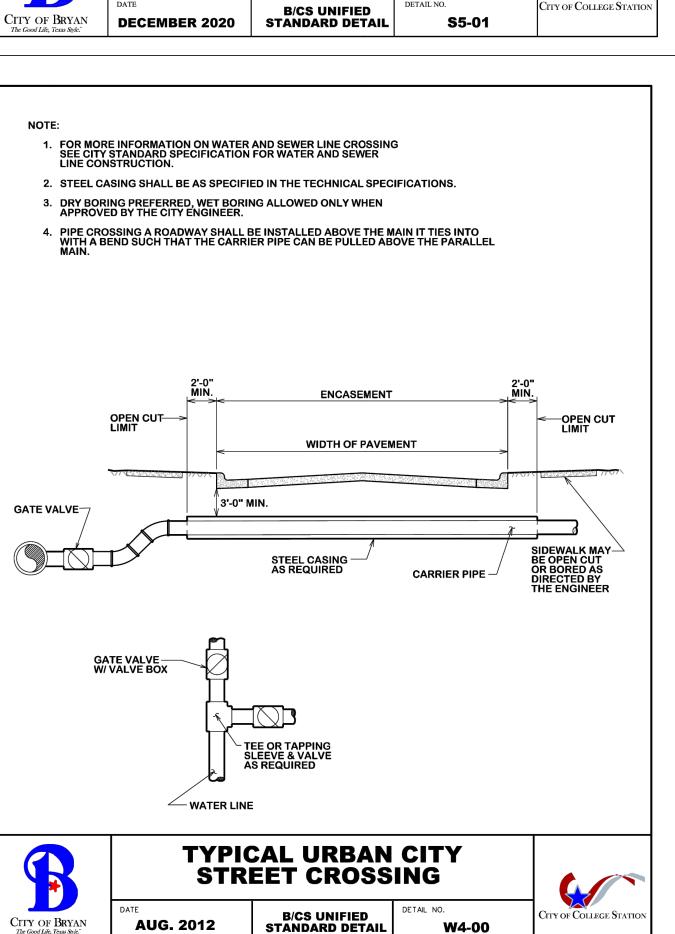
**PAVED AREAS** 

STANDARD DETAIL

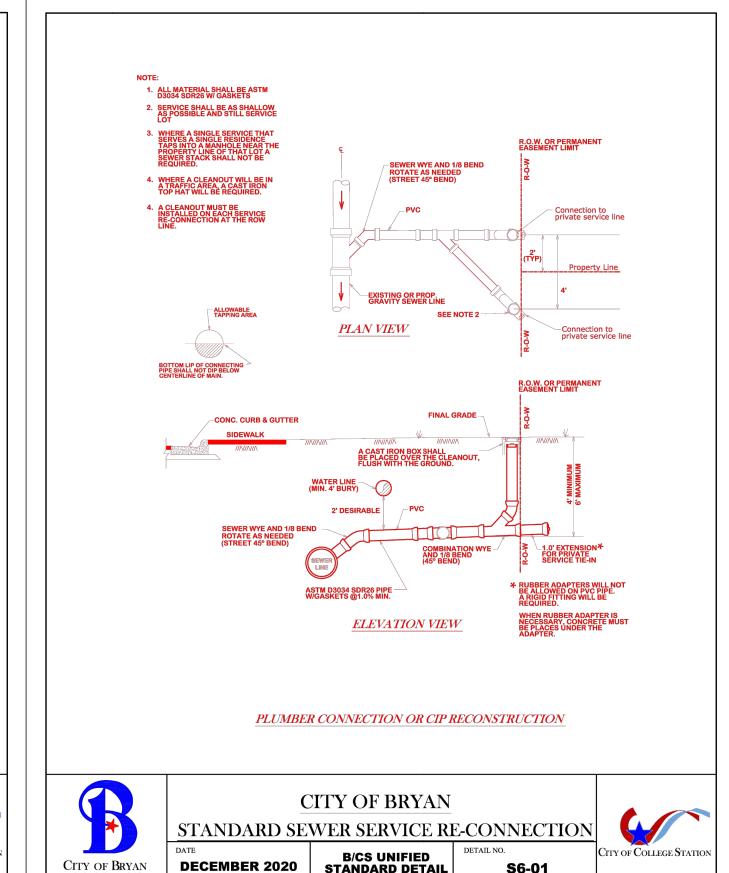


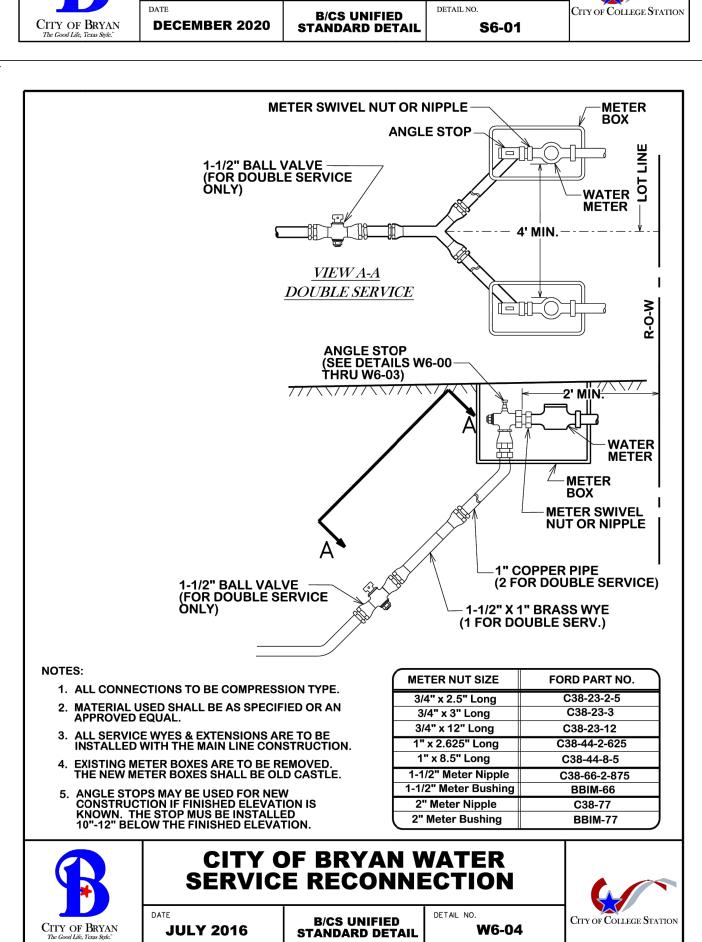






W4-00





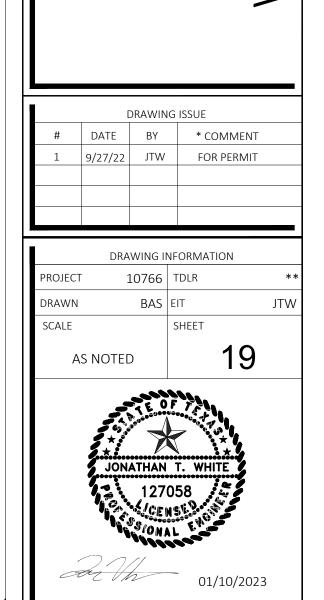


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\*PLANS NOT RELEASED FOR CONSTRUCTION LINLESS INDICATED ABOV

#### PROJECT SUMMARY

#### CALCULATION DETAILS

- LOADING = HS20/HS25
- APPROX. LINEAR FOOTAGE = 334 LF

#### STORAGE SUMMARY

- STORAGE VOLUME REQUIRED = 6,534 CF
- PIPE STORAGE VOLUME = 6,548 CF
- BACKFILL STORAGE VOLUME = 0 CF
- TOTAL STORAGE PROVIDED = 6,548 CF

#### PIPE DETAILS

- DIAMETER = 60"
- CORRUGATION = 5x1
- GAGE = 16
- COATING = ALT2
- WALL TYPE = SOLID
- BARREL SPACING = 18"

#### **BACKFILL DETAILS**

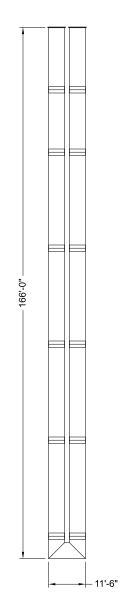
- WIDTH AT ENDS = 12"
- ABOVE PIPE = 0"
- WIDTH AT SIDES = 12"
- BELOW PIPE = 0"

#### **NOTES**

- ALL RISER AND STUB DIMENSIONS ARE TO CENTERLINE. ALL ELEVATIONS, DIMENSIONS, AND LOCATIONS OF RISERS AND INLETS, SHALL BE VERIFIED BY THE ENGINEER OF RECORD PRIOR TO RELEASING FOR FABRICATION.
- ALL FITTINGS AND REINFORCEMENT COMPLY WITH ASTM A998.
- ALL RISERS AND STUBS ARE  $2\frac{2}{3}$ " x  $\frac{1}{2}$ " CORRUGATION AND 16 GAGE UNLESS OTHERWISE NOTED.
- RISERS TO BE FIELD TRIMMED TO GRADE.
- QUANTITY OF PIPE SHOWN DOES NOT PROVIDE EXTRA PIPE FOR CONNECTING THE SYSTEM TO EXISTING PIPE OR DRAINAGE STRUCTURES. OUR SYSTEM AS DETAILED PROVIDES NOMINAL INLET AND/OR OUTLET PIPE STUB FOR CONNECTION TO EXISTING DRAINAGE FACILITIES. IF ADDITIONAL PIPE IS NEEDED IT IS THE RESPONSIBILITY OF THE CONTRACTOR.
- BAND TYPE TO BE DETERMINED UPON FINAL DESIGN.
- THE PROJECT SUMMARY IS REFLECTIVE OF THE DYODS DESIGN, QUANTITIES ARE APPROX. AND SHOULD BE VERIFIED UPON FINAL DESIGN AND APPROVAL. FOR EXAMPLE, TOTAL EXCAVATION DOES NOT CONSIDER ALL VARIABLES SUCH AS SHORING AND ONLY ACCOUNTS FOR MATERIAL WITHIN THE ESTIMATED EXCAVATION FOOTPRINT.
- THESE DRAWINGS ARE FOR CONCEPTUAL PURPOSES AND DO NOT REFLECT ANY LOCAL PREFERENCES OR REGULATIONS. PLEASE CONTACT YOUR LOCAL CONTECH REP FOR MODIFICATIONS.

DATE

If discrepancies between the supplied information upon which the drawing is based and actual field conditions are encountered as site work progresses, these discrepancies must be reported to Contech immediately for re-evaluation of the design. Contech accepts no liability for designs based on missing, incomplete or inaccurate information supplied by others.



**ASSEMBLY** SCALE: 1" = 30'

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REVISION DESCRIPTION	BY	800-338-1122 513-645-7000 513-645-799

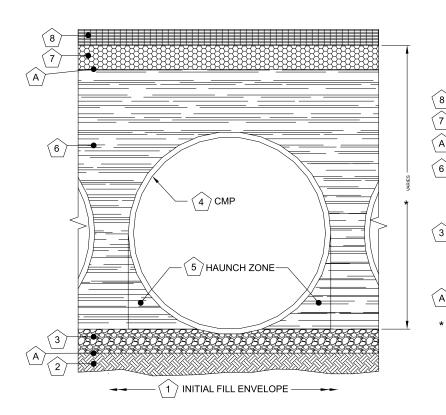


DYO20336 The Lux 60" diameter Solid CMP Bryan, TX **DETENTION SYSTEM** 

PROJECT No.:	SEQ. I	No.:	DATE:
13173	20336		8/17/2022
DESIGNED:		DRAW	N:
DYO			DYO
CHECKED:		APPR	OVED:
DYO			DYO
SHEET NO.:			
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Material Location	Description	Material Designation	Designation
Rigid or Flexible Pa (if applicable)	vement		
Road Base (if applicable)			
Geotextile Layer	Non-Woven Geotextile	CONTECH C-40 or C-45	Engineer Decision for consideration to prevent so migration into varying soil types
Backfill	Well graded granular material which may contain small amounts of silt or clay.	AASHTO M 145- A-1, A-2, A-3	Placed in 8" +/- loose lifts and compacted to 90% Standard Proctor Per AASHTO T 99
Bedding Stone	Well graded granular bedding material w/maximum particle size of 3"	AASHTO M43 - 3,357,4,467, 5, 56, 57	Engineer to determine if bedding is required. Pipe may be placed on the trench bottom of a relatively loose, native suitable well graded & granular material. For Arch pipes it is recommended to be shaped to a relatively flat bottom or fine-grade the foundation to a slight v-shape. Unsuitable material should be over-excavated and re-placed with a 4"-6" layer of well graded & granular stone per the material designation. See AASHTO 26.3.8.1 / 26.5.3 Bedding info.
Geotextile Layer	Non-Woven Geotextile	CONTECH C-40 or C-45	Engineer Decision for consideration to prevent so migration into varying soil types

Note: Backfill using controlled low-strength material (CLSM, "flash fill" or "flowable fill") when the spacing between the pipes will

not allow for placement and adequate compaction of the backfill.

MINIMUM WIDTH DEPENDS ON SITE CONDITIONS AND ENGINEERING JUDGEMENT

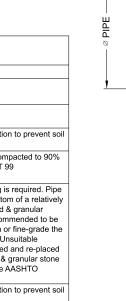
#### FOUNDATION/BEDDING PREPARATION

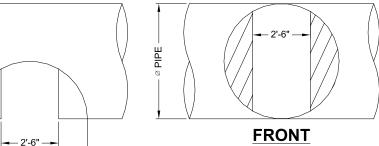
- PRIOR TO PLACING THE BEDDING, THE FOUNDATION MUST BE CONSTRUCTED TO A UNIFORM AND STABLE GRADE. IN THE EVENT THAT UNSUITABLE FOUNDATION MATERIALS ARE ENCOUNTERED DURING EXCAVATION, THEY SHALL BE REMOVED AND BROUGHT BACK TO THE GRADE WITH A FILL MATERIAL AS APPROVED BY THE ENGINEER.
- HAUNCH ZONE MATERIAL SHALL BE PLACED AND UNIFORMALLY COMPACTED WITHOUT

#### **BACKFILL**

WHEN PLACING THE FIRST LIFTS OF BACKFILL IT IS IMPORTANT TO MAKE SURE THAT THE BACKFILL IS PROPERLY COMPACTED UNDER AND AROUND THE PIPE HAUNCHES. BACKFILL SHALL BE PLACED SUCH THAT THERE IS NO MORE THAN A TWO LIFT (16") DIFFERENTIAL BETWEEN ANY OF THE PIPES AT ANY TIME DURING THE BACKFILL PROCESS. THE BACKFILL SHALL BE ADVANCED ALONG THE LENGTH OF THE DETENTION SYSTEM AT THE SAME RATE TO AVOID DIFFERENTIAL LOADING ON THE PIPE.

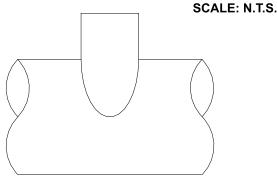
OTHER ALTERNATE BACKFILL MATERIAL MAY BE ALLOWED DEPENDING ON SITE SPECIFIC CONDITIONS, AS APPROVED BY SITE ENGINEER.





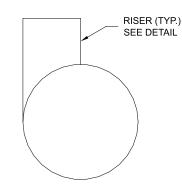
MANWAY DETAIL APPLICABLE FOR CMP SYSTEMS WITH DIAMETERS 48" AND LARGER. MANWAYS MAY BE REQUIRED ON SMALLER SYSTEMS DEPENDING ON ACTUAL SITE SPECIFIC CONDITIONS.

#### **TYPICAL MANWAY DETAIL**



Ø PIPF

**PLAN** 



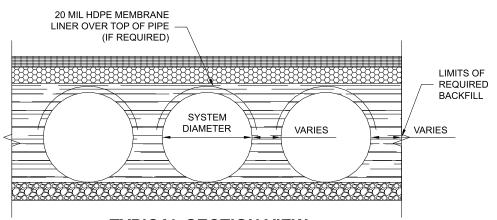
#### **ELEVATION**

#### TYPICAL RISER DETAIL

SCALE: N.T.S.

**END** 

LADDERS ARE OPTIONAL AND ARE NOT REQUIRED FOR ALL SYSTEMS.



#### **TYPICAL SECTION VIEW**

**LINER OVER ROWS** SCALE: N.T.S.

**NOTE:** IF SALTING AGENTS FOR SNOW AND ICE REMOVAL ARE USED ON OR NEAR THE PROJECT, AN HDPE MEMBRANE LINER IS RECOMMENDED WITH THE SYSTEM. THE IMPERMEABLE LINER IS INTENDED TO HELP PROTECT THE SYSTEM FROM THE POTENTIAL ADVERSE EFFECTS THAT MAY RESULT FROM A CHANGE IN THE SURROUNDING ENVIRONMENT OVER A PERIOD OF TIME. PLEASE REFER TO THE CORRUGATED METAL PIPE DETENTION DESIGN GUIDE FOR ADDITIONAL INFORMATION.

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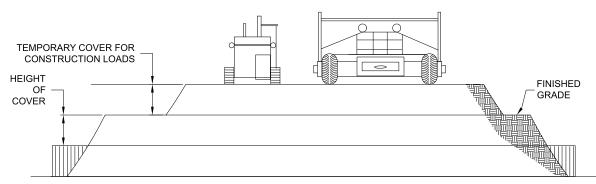
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**CHATECH** CMP DETENTION SYSTEMS **DYODS** 

DRAWING

DYO20336 The Lux 60" diameter Solid CMP Bryan, TX **DETENTION SYSTEM** 

PROJECT No.:	SEQ. I	No.:	DATE:
13173	203	336	8/17/2022
DESIGNED:		DRAW	/N:
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CHECKED:		APPR	OVED:
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SHEET NO.:			
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#### **CONSTRUCTION LOADS**

FOR TEMPORARY CONSTRUCTION VEHICLE LOADS, AN EXTRA AMOUNT OF COMPACTED COVER MAY BE REQUIRED OVER THE TOP OF THE PIPE. THE HEIGHT-OF-COVER SHALL MEET THE MINIMUM REQUIREMENTS SHOWN IN THE TABLE BELOW. THE USE OF HEAVY CONSTRUCTION EQUIPMENT NECESSITATES GREATER PROTECTION FOR THE PIPE THAN FINISHED GRADE COVER MINIMUMS FOR NORMAL HIGHWAY TRAFFIC.

PIPE SPAN, INCHES	AXLE LOADS (kips)							
INCHES	18-50	50-75	75-110	110-150				
	MINIMUM COVER (FT)							
12-42	2.0	2.5	3.0	3.0				
48-72	3.0	3.0	3.5	4.0				
78-120	3.0	3.5	4.0	4.0				
126-144	3.5	4.0	4.5	4.5				

\*MINIMUM COVER MAY VARY, DEPENDING ON LOCAL CONDITIONS. THE CONTRACTOR MUST PROVIDE THE ADDITIONAL COVER REQUIRED TO AVOID DAMAGE TO THE PIPE. MINIMUM COVER IS MEASURED FROM THE TOP OF THE PIPE TO THE TOP OF THE MAINTAINED CONSTRUCTION ROADWAY SURFACE.

#### **CONSTRUCTION LOADING DIAGRAM**

SCALE: N.T.S.

#### SPECIFICATION FOR DESIGNED DETENTION SYSTEM:

#### SCOPE

THIS SPECIFICATION COVERS THE MANUFACTURE AND INSTALLATION OF THE DESIGNED DETENTION SYSTEM DETAILED IN THE PROJECT PLANS.

#### **MATERIA**

WATERVIAL
THE MATERIAL SHALL CONFORM TO THE APPLICABLE REQUIREMENTS LISTED BELOW:

ALUMINIZED TYPE 2 STEEL COILS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M-274 OR ASTM A-92.

THE GALVANIZED STEEL COILS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M-218 OR ASTM A-929.

THE POLYMER COATED STEEL COILS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M-246 OR ASTM A-742.

THE ALUMINUM COILS SHALL CONFORM TO THE APPLICABLE OF AASHTO M-197 OR ASTM B-744.

#### **CONSTRUCTION LOADS**

THESE DRAWINGS ARE FOR CONCEPTUAL PURPOSES AND DO NOT REFLECT ANY LOCAL

PREFERENCES OR REGULATIONS. PLEASE

CONSTRUCTION LOADS MAY BE HIGHER THAN FINAL LOADS. FOLLOW THE MANUFACTURER'S OR NCSPA GUIDELINES.

#### DIDE

THE PIPE SHALL BE MANUFACTURED IN ACCORDANCE TO THE APPLICABLE REQUIREMENTS LISTED BELOW:

ALUMINIZED TYPE 2: AASHTO M-36 OR ASTM A-760

GALVANIZED: AASHTO M-36 OR ASTM A-760

AFPOLYMELE COATED: AASHTO M-245 OR ASTM A-762

ALUMINUM: AASHTO M-196 OR ASTM B-745 APPLICABLE

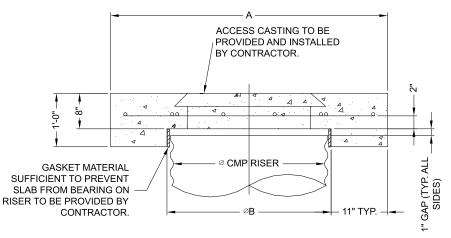
#### HANDLING AND ASSEMBLY

SHALL BE IN ACCORDANCE WITH NCSP'S (NATIONAL CORRUGATED STEEL APPRECABSDICIATION) FOR ALUMINIZED TYPE 2, GALVANIZED OR POLYMER COATED STEEL. SHALL BE IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS FOR ALUMINUM PIPE.

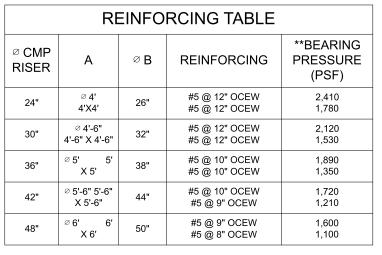
#### REQUIREMENTS INSTALLATION

SHALL BE IN ACCORDANCE WITH AASHTO STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES, SECTION 26, DIVISION II DIVISION II OR ASTM A-798 (FOR ALUMINIZED TYPE 2, GALVANIZED OR POLYMER COATED STEEL) OR ASTM B-788 (FOR ALUMINUM PIPE) AND IN CONFORMANCE WITH THE PROJECT PLANS AND SPECIFICATIONS. IF THERE ARE ANY INCONSISTENCIES OR CONFLICTS THE CONTRACTOR SHOULD DISCUSS AND RESOLVE WITH THE SITE ENGINEER.

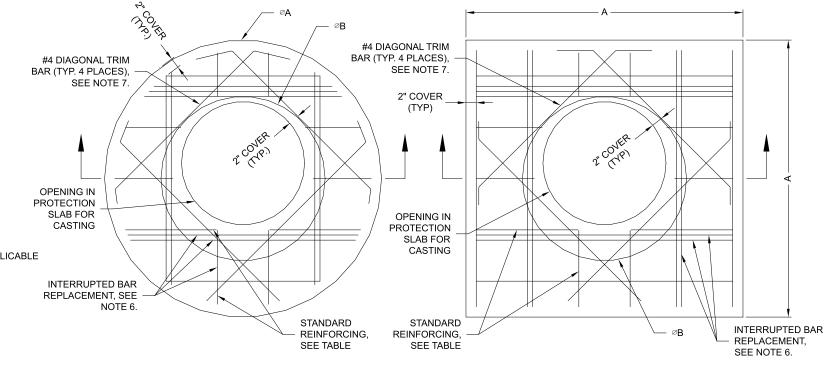
IT IS ALWAYS THE RESPONSIBILITY OF THE CONTRACTOR TO FOLLOW OSHA GUIDELINES FOR SAFE PRACTICES.



#### **SECTION VIEW**



\*\* ASSUMED SOIL BEARING CAPACITY



#### ROUND OPTION PLAN VIEW

#### NOTES:

- 1. DESIGN IN ACCORDANCE WITH AASHTO, 17th EDITION.
- 2. DESIGN LOAD HS25.
- 3. EARTH COVER = 1' MAX.
- 4. CONCRETE STRENGTH = 3,500 psi
- 5. REINFORCING STEEL = ASTM A615, GRADE 60.
- PROVIDE ADDITIONAL REINFORCING AROUND OPENINGS EQUAL TO THE BARS INTERRUPTED, HALF EACH SIDE. ADDITIONAL BARS TO BE IN THE SAME PLANE.

#### SQUARE OPTION PLAN VIEW

- 7. TRIM OPENING WITH DIAGONAL #4 BARS, EXTEND BARS A MINIMUM OF 12" BEYOND OPENING, BEND BARS AS REQUIRED TO MAINTAIN BAR COVER.
- 8. PROTECTION SLAB AND ALL MATERIALS TO BE PROVIDED AND INSTALLED BY CONTRACTOR.
- 9. DETAIL DESIGN BY DELTA ENGINEERING, BINGHAMTON, NY.

#### **MANHOLE CAP DETAIL**

SCALE: N.T.S.

CONTACT YOUR LOCAL CONTECH REP FOR
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DYO20336 The Lux 60" diameter Solid CMP Bryan, TX DETENTION SYSTEM

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13173	203	336	8/17/2022
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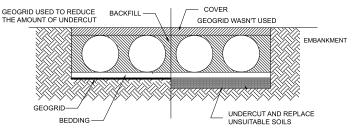
#### **CMP DETENTION INSTALLATION GUIDE**

PROPER INSTALLATION OF A FLEXIBLE UNDERGROUND DETENTION SYSTEM WILL ENSURE LONG-TERM PERFORMANCE. THE CONFIGURATION OF THESE SYSTEMS OFTEN REQUIRES SPECIAL CONSTRUCTION PRACTICES THAT DIFFER FROM CONVENTIONAL FLEXIBLE PIPE CONSTRUCTION. CONTECH ENGINEERED SOLUTIONS STRONGLY SUGGESTS SCHEDULING A PRE-CONSTRUCTION MEETING WITH YOUR LOCAL SALES ENGINEER TO DETERMINE IF ADDITIONAL MEASURES, NOT COVERED IN THIS GUIDE, ARE APPROPRIATE FOR YOUR SITE.

#### **FOUNDATION**

CONSTRUCT A FOUNDATION THAT CAN SUPPORT THE DESIGN LOADING APPLIED BY THE PIPE AND ADJACENT BACKFILL WEIGHT AS WELL AS MAINTAIN ITS INTEGRITY DURING CONSTRUCTION.

IF SOFT OR UNSUITABLE SOILS ARE ENCOUNTERED, REMOVE THE POOR SOILS DOWN TO A SUITABLE DEPTH AND THEN BUILD UP TO THE APPROPRIATE FLEVATION WITH A COMPETENT BACKELL MATERIAL. THE STRUCTURAL FILL MATERIAL GRADATION SHOULD NOT ALLOW THE MIGRATION OF FINES, WHICH CAN CAUSE SETTLEMENT OF THE DETENTION SYSTEM OR PAVEMENT ABOVE. IF THE STRUCTURAL FILL MATERIAL IS NOT COMPATIBLE WITH THE UNDERLYING SOILS AN ENGINEERING FABRIC SHOULD BE USED AS A SEPARATOR IN SOME CASES LISING A STIFF REINFORCING GEOGRIC REDUCES OVER EXCAVATION AND REPLACEMENT FILL QUANTITIES.

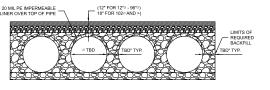


GRADE THE FOUNDATION SUBGRADE TO A UNIFORM OR SLIGHTLY SLOPING GRADE. IF THE SUBGRADE IS CLAY OR RELATIVELY NON-POROUS AND THE CONSTRUCTION SEQUENCE WILL LAST FOR AN EXTENDED PERIOD OF TIME. IT IS BEST TO SLOPE THE GRADE TO ONE END OF THE SYSTEM. THIS WILL ALLOW EXCESS WATER TO DRAIN QUICKLY, PREVENTING SATURATION OF THE SUBGRADE

#### **GEOMEMBRANE BARRIER**

A SITE'S RESISTIVITY MAY CHANGE OVER TIME WHEN VARIOUS TYPES OF SALTING AGENTS ARE USED, SUCH AS ROAD SALTS FOR DEICING AGENTS. IF SALTING AGENTS ARE USED ON OR NEAR THE PROJECT SITE, A GEOMEMBRANE BARRIER IS RECOMMENDED WITH THE SYSTEM. THE GEOMEMBRANE LINER IS INTENDED TO HELP PROTECT THE SYSTEM FROM THE POTENTIAL ADVERSE EFFECTS THAT MAY RESULT FROM THE USE OF SUCH AGENTS INCLUDING PREMATURE CORROSION AND REDUCED ACTUAL SERVICE LIFE.

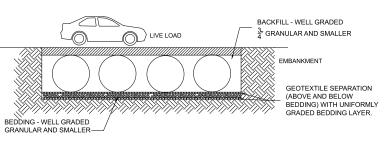
THE PROJECT'S ENGINEER OF RECORD IS TO EVALUATE WHETHER SALTING AGENTS WILL BE USED ON OR NEAR THE PROJECT SITE, AND USE HIS/HER BEST JUDGEMENT TO DETERMINE IF ANY ADDITIONAL PROTECTIVE MEASURES ARE REQUIRED. BELOW IS A TYPICAL DETAIL SHOWING THE PLACEMENT OF A GEOMEMBRANE BARRIER FOR PROJECTS WHERE SALTING AGENTS ARE USED ON OR NEAR THE PROJECT SITE



#### **IN-SITU TRENCH WALL**

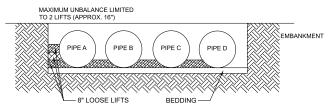
IF EXCAVATION IS REQUIRED, THE TRENCH WALL NEEDS TO BE CAPABLE OF SUPPORTING THE LOAD THAT THE PIPE SHEDS AS THE SYSTEM IS LOADED. IF SOILS ARE NOT CAPABLE OF SUPPORTING THESE LOADS, THE PIPE CAN DEFLECT PERFORM A SIMPLE SOIL PRESSURE CHECK USING THE APPLIED LOADS TO DETERMINE THE LIMITS OF EXCAVATION BEYOND THE SPRING LINE OF THE **OUTER MOST PIPES** 

IN MOST CASES THE REQUIREMENTS FOR A SAFE WORK ENVIRONMENT AND PROPER BACKFILL PLACEMENT AND COMPACTION TAKE CARE OF THIS CONCERN.



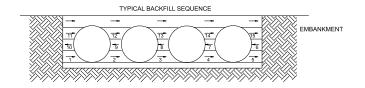
#### **BACKFILL PLACEMENT**

MATERIAL SHALL BE WORKED INTO THE PIPE HAUNCHES BY MEANS OF SHOVEL-SLICING, RODDING, AIR TAMPER, VIBRATORY ROD, OR OTHER EFFECTIVE **METHODS** 

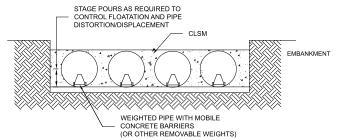


IF AASHTO T99 PROCEDURES ARE DETERMINED INFEASIBLE BY THE GEOTECHNICAL ENGINEER OF RECORD, COMPACTION IS CONSIDERED ADEQUATE WHEN NO FURTHER YIELDING OF THE MATERIAL IS OBSERVED UNDER THE COMPACTOR, OR UNDER FOOT, AND THE GEOTECHNICAL ENGINEER OF RECORD (OR REPRESENTATIVE THEREOF) IS SATISFIED WITH THE LEVEL OF COMPACTION.

FOR LARGE SYSTEMS, CONVEYOR SYSTEMS, BACKHOES WITH LONG REACHES OR DRAGLINES WITH STONE BUCKETS MAY BE USED TO PLACE BACKFILL. ONCE MINIMUM COVER FOR CONSTRUCTION LOADING ACROSS THE ENTIRE WIDTH OF THE SYSTEM IS REACHED. ADVANCE THE EQUIPMENT TO THE END OF THE RECENTLY PLACED FILL, AND BEGIN THE SEQUENCE AGAIN UNTIL THE SYSTEM IS COMPLETELY BACKFILLED. THIS TYPE OF CONSTRUCTION SEQUENCE PROVIDES ROOM FOR STOCKPILED BACKFILL DIRECTLY BEHIND THE BACKHOE AS WELL AS THE MOVEMENT OF CONSTRUCTION TRAFFIC. MATERIAL STOCKPILES ON TOP OF THE BACKFILLED DETENTION SYSTEM SHOULD BE LIMITED TO 8- TO 10-FEET HIGH AND MUST PROVIDE BALANCED LOADING ACROSS ALL BARRELS. TO DETERMINE THE PROPER COVER OVER THE PIPES TO ALLOW THE MOVEMENT OF CONSTRUCTION EQUIPMENT SEE TABLE 1, OR CONTACT YOUR LOCAL CONTECH SALES ENGINEER.



WHEN FLOWABLE FILL IS USED, YOU MUST PREVENT PIPE FLOATATION TYPICALLY, SMALL LIFTS ARE PLACED BETWEEN THE PIPES AND THEN ALLOWED TO SET-UP PRIOR TO THE PLACEMENT OF THE NEXT LIFT. THE ALLOWABLE THICKNESS OF THE CLSM LIFT IS A FUNCTION OF A PROPER BALANCE BETWEEN THE UPLIFT FORCE OF THE CLSM, THE OPPOSING WEIGHT OF THE PIPE, AND THE EFFECT OF OTHER RESTRAINING MEASURES. THE PIPE CAN CARRY LIMITED FLUID PRESSURE WITHOUT PIPE DISTORTION OR DISPLACEMENT, WHICH ALSO AFFECTS THE CLSM LIFT THICKNESS. YOUR LOCAL CONTECH SALES ENGINEER CAN HELP DETERMINE THE PROPER LIFT THICKNESS.

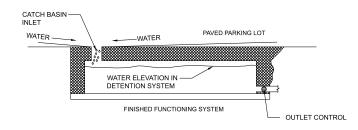


#### **CONSTRUCTION LOADING**

TYPICALLY, THE MINIMUM COVER SPECIFIED FOR A PROJECT ASSUMES H-20 LIVE LOAD. BECAUSE CONSTRUCTION LOADS OFTEN EXCEED DESIGN LIVE LOADS, INCREASED TEMPORARY MINIMUM COVER REQUIREMENTS ARE NECESSARY. SINCE CONSTRUCTION EQUIPMENT VARIES FROM JOB TO JOB, IT IS BEST TO ADDRESS EQUIPMENT SPECIFIC MINIMUM COVER REQUIREMENTS WITH YOUR LOCAL CONTECH SALES ENGINEER DURING YOUR PRE-CONSTRUCTION MEETING.

#### **ADDITIONAL CONSIDERATIONS**

BECAUSE MOST SYSTEMS ARE CONSTRUCTED BELOW-GRADE, RAINFALL CAN RAPIDLY FILL THE EXCAVATION; POTENTIALLY CAUSING FLOATATION AND MOVEMENT OF THE PREVIOUSLY PLACED PIPES. TO HELP MITIGATE POTENTIAL PROBLEMS, IT IS BEST TO START THE INSTALLATION AT THE DOWNSTREAM END WITH THE OUTLET ALREADY CONSTRUCTED TO ALLOW A ROUTE FOR THE WATER TO ESCAPE. TEMPORARY DIVERSION MEASURES MAY BE REQUIRED FOR HIGH FLOWS DUE TO THE RESTRICTED NATURE OF THE OUTLET PIPE



#### **CMP DETENTION SYSTEM INSPECTION AND MAINTENANCE**

UNDERGROUND STORMWATER DETENTION AND INFILTRATION SYSTEMS MUST BE INSPECTED AND MAINTAINED AT REGULAR INTERVALS FOR PURPOSES OF PERFORMANCE AND LONGEVITY.

#### INSPECTION

INSPECTION IS THE KEY TO EFFECTIVE MAINTENANCE OF CMP DETENTION SYSTEMS AND IS EASILY PERFORMED. CONTECH RECOMMENDS ONGOING. ANNUAL INSPECTIONS. SITES WITH HIGH TRASH LOAD OR SMALL OUTLET CONTROL ORIFICES MAY NEED MORE FREQUENT INSPECTIONS. THE RATE AT WHICH THE SYSTEM COLLECTS POLLUTANTS WILL DEPEND MORE ON SITE SPECIFIC ACTIVITIES RATHER THAN THE SIZE OR CONFIGURATION OF THE SYSTEM.

INSPECTIONS SHOULD BE PERFORMED MORE OFTEN IN EQUIPMENT WASHDOWN AREAS, IN CLIMATES WHERE SANDING AND/OR SALTING OPERATIONS TAKE PLACE, AND IN OTHER VARIOUS INSTANCES IN WHICH ONE WOULD EXPECT HIGHER ACCUMULATIONS OF SEDIMENT OR ABRASIVE/ CORROSIVE CONDITIONS. A RECORD OF EACH INSPECTION IS TO BE MAINTAINED FOR THE LIFE OF THE SYSTEM

#### MAINTENANCE

CMP DETENTION SYSTEMS SHOULD BE CLEANED WHEN AN INSPECTION REVEALS ACCUMULATED SEDIMENT OR TRASH IS CLOGGING THE DISCHARGE

ACCUMULATED SEDIMENT AND TRASH CAN TYPICALLY BE EVACUATED. THROUGH THE MANHOLE OVER THE OUTLET ORIFICE. IF MAINTENANCE IS NOT PERFORMED AS RECOMMENDED, SEDIMENT AND TRASH MAY ACCUMULATE IN FRONT OF THE OUTLET ORIFICE. MANHOLE COVERS SHOULD BE SECURELY SEATED FOLLOWING CLEANING ACTIVITIES. CONTECH SUGGESTS THAT ALL SYSTEMS BE DESIGNED WITH AN ACCESS/INSPECTION MANHOLE SITUATED AT OR NEAR THE INLET AND THE OUTLET ORIFICE. SHOULD IT BE NECESSARY TO GET INSIDE THE SYSTEM TO PERFORM MAINTENANCE ACTIVITIES, ALL APPROPRIATE PRECAUTIONS REGARDING CONFINED SPACE ENTRY AND OSHA REGULATIONS SHOULD BE FOLLOWED.

ANNUAL INSPECTIONS ARE BEST PRACTICE FOR ALL UNDERGROUND SYSTEMS. DURING THIS INSPECTION, IF EVIDENCE OF SALTING/DE-ICING AGENTS IS OBSERVED WITHIN THE SYSTEM, IT IS BEST PRACTICE FOR THE SYSTEM TO BE RINSED, INCLUDING ABOVE THE SPRING LINE SOON AFTER THE SPRING THAW AS PART OF THE MAINTENANCE PROGRAM FOR THE SYSTEM

MAINTAINING AN UNDERGROUND DETENTION OR INFILTRATION SYSTEM IS EASIEST WHEN THERE IS NO FLOW ENTERING THE SYSTEM. FOR THIS REASON, IT IS A GOOD IDEA TO SCHEDULE THE CLEANOUT DURING DRY

THE FOREGOING INSPECTION AND MAINTENANCE EFFORTS HELP ENSURE UNDERGROUND PIPE SYSTEMS USED FOR STORMWATER STORAGE CONTINUE TO FUNCTION AS INTENDED BY IDENTIFYING RECOMMENDED REGULAR INSPECTION AND MAINTENANCE PRACTICES. INSPECTION AND MAINTENANCE RELATED TO THE STRUCTURAL INTEGRITY OF THE PIPE OR THE SOUNDNESS OF PIPE JOINT CONNECTIONS IS BEYOND THE SCOPE OF THIS GUIDE.

V8.D					
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DRAWING

DYO20336 The Lux 60" diameter Solid CMP Bryan, TX **DETENTION SYSTEM** 

PROJECT No.: 13173	SEQ. No.: 20336		DATE: 8/17/2022	
DESIGNED:		DRAW	'N:	
DYO			DYO	
CHECKED:		APPROVED:		
DYO			DYO	
SHEET NO.:			1	

LEFT UNIT (2335 SF) 120/240V, 1Ø, 3W DESCRIPTION SHTING/SMALL APPLIANCE VA LIGHTING/RECEPT 3 VA/SQ. FT. LAUNDRY WTR HTR DRYER OVEN/RANGE TOTAL KVA = 46.0 KITCHEN SMALL APPLIANCE 3000 TOTAL GENERAL LOAD VA 46005 KVA DF L1 L2
FIRST 10 KW @ 100% 10000 10.0 1.00 42 42
REMAINDER @ 40% 36005 36.0 0.4 60 60
HVAC (5 KVA / 1000 SQ FT) 11.7 1.00 49 49
TOTAL DEMAND LOAD 151 151 TOTAL DEMAND LOAD

ESTIMATED ELECT. LOAD SUMMARY

ESTIMATED ELECT. LO HOUSE PANEL (AT E 120/240V, 1Ø,	ACH			
DESCRIPTION	KVA	DF	L1	L2
LIGHTING	2.0	1.25	10	10
RECEPT	2.0	1.00	8	8
MISC. LOAD (EST)	5.0	1.00	21	2
TOTAL DEMAND LOAD	9 KVA	-	39	39

	ESTIMATED ELECT. LOAD SUMMARY								
	CENTER LEFT UNIT (2063 SF)								
	120/	/240V, 1Ø,	3W						
	DESCRIPTION								
LIC	GHTING/SMALL APPLIANCE	VA							
	LIGHTING/RECEPT								
1	3 VA/SQ. FT.	6189							
1	LAUNDRY	3000							
ı	WTR HTR	4500							
ı	DRYER	10000							
	OVEN/RANGE	12000	TOTAL	KVA = 38.7	,				
	KITCHEN SMALL APPLIANCE	3000							
	TOTAL GENERAL LOAD VA	38689	KVA	DF	L1	L2			
	FIRST 10 KW @ 100%	10000	10.0	1.00	42	4			
	REMAINDER @ 40%	28689	28.7	0.4	48	4			
Η	/AC (5 KVA / 1000 SQ FT)		10.3	1.00	43	4			
T	OTAL DEMAND LOAD				133	13			

ELECTRICAL SERVICE METER LOAD:

13 BUILDINGS, EACH WITH:

1 CENTER LEFT UNIT 1 CENTER RIGHT UNIT

1 LEFT UNIT

1 RIGHT UNIT

1 HOUSE PANEL

120/	240V, 1Ø,	3W			
DESCRIPTION					
GHTING/SMALL APPLIANCE	VA				
LIGHTING/RECEPT					
3 VA/SQ. FT.	6189				
LAUNDRY	3000				
WTR HTR	4500				
DRYER	10000				
OVEN/RANGE	12000	TOTAL	KVA = 38.7	7	
KITCHEN SMALL APPLIANCE	3000	.01/12			
TOTAL GENERAL LOAD VA	38689	KVA	DF	L1	L2
FIRST 10 KW @ 100%	10000	10.0	1.00	42	42
REMAINDER @ 40%	28689	28.7	0.4	48	48
VAC (5 KVA / 1000 SQ FT)		10.3	1.00	43	43
OTAL DEMAND LOAD				133	133

ESTIMATED ELECT. LOAD SUMMARY								
PER	BUIL	DING						
120/240V, 1Ø, 3W								
DESCRIPTION	VA	KVA	DF	L1	L2			
TOTAL GENERAL LOAD VA	169286							
FIRST 10 KW/UNIT @ 100%	40000	40.0	1.00	167	167			
REMAINDER @ 40%	129286	129.3	0.4	216	216			
HVAC (5 KVA / 1000 SQ FT)		43.8	1.00	183	183			
HOUSE PANEL LIGHTING		2.0	1.25	10	10			
HOUSE PANEL RECEPT		2.0	1.00	8	8			
HOUSE PANEL MISC. LOAD (EST)		5.0	1.00	21	21			
TOTAL DEMAND LOAD				605	605			

ESTIMATED ELECT. LOAD SUMMARY

CENTER RIGHT UNIT (2279 SF)

TOTAL GENERAL LOAD VA 45837 KVA DF L1 L2
FIRST 10 KW @ 100% 10000 10.0 1.00 42 42
REMAINDER @ 40% 35837 35.8 0.4 60 60
HVAC (5 KVA / 1000 SQ FT) 11.4 1.00 48 48
TOTAL DEMAND LOAD 150

12000 TOTAL KVA = 45.8

DESCRIPTION

3 VA/SQ. FT.

OVEN/RANGE

TOTAL DEMAND LOAD

LAUNDRY WTR HTR

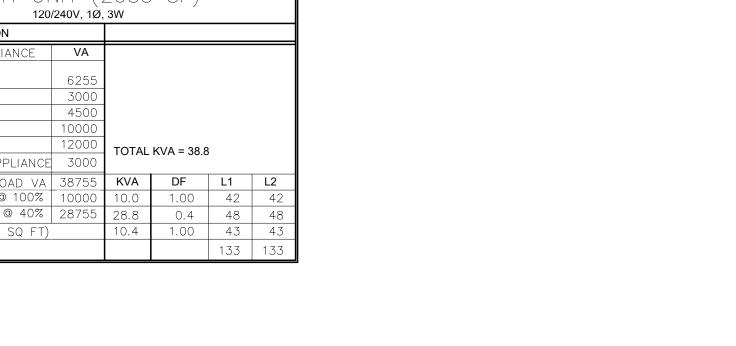
DRYER

SHTING/SMALL APPLIANCE VA LIGHTING/RECEPT

KITCHEN SMALL APPLIANCE 3000

HVAC (5 KVA / 1000 SQ FT)

ESTIMATED ELECT. LOAD SUMMARY							
right unit (2085 sf)							
120/240V, 1Ø, 3W							
DESCRIPTION							
LIGHTING/SMALL APPLIANCE	VA						
LIGHTING/RECEPT							
3 VA/SQ. FT.	6255						
LAUNDRY	3000						
WTR HTR	4500						
DRYER	10000						
OVEN/RANGE	12000	ΤΟΤΔΙ	K\/Δ = 38.8	1			
KITCHEN SMALL APPLIANCE	3000	TOTAL	OTAL KVA = 38.8				
TOTAL GENERAL LOAD VA	38755	KVA	DF	L1	L2		
FIRST 10 KW @ 100%	10000	10.0	1.00	42	42		
REMAINDER @ 40%	28755	28.8	0.4	48	48		
HVAC (5 KVA / 1000 SQ FT)		10.4	1.00	43	43		
TOTAL DEMAND LOAD				133	133		





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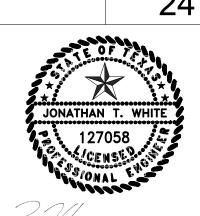
21123 EVA STREET #200 MONTGOMERY, TEXAS 77356

CLIENT INFORMATION
JUSTIN WALTON 713-446-4083 JW@BUILD-OLOGY.NET

PROJECT ADDRESS 2539 E VILLA MARIA ROAD BRYAN, TEXAS 77803

DRAWING ISSUE				
#	DATE	BY	* COMMENT	
1	9/27/22	JTW	FOR PERMIT	

П	DRAWING INFORMATION				
	PROJECT	10766	TDLR	k	
	DRAWN	BAS	EIT	JTV	
	SCALE		SHEET		
				24	



01/10/2023